## SEARS BUILDING REMODEL PROJECT CITY OF SANTA MARIA, CALIFORNIA

# TRAFFIC, CIRCULATION AND VMT STUDY



April 19, 2024

ATE #24017

Mark Gabay Charles Company 9034 W. Sunset Boulevard West Hollywood, CA 90069





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April 19, 2024

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#### TRAFFIC, CIRCULATION AND VMT STUDY FOR THE SEARS BUILDING REMODEL PROJECT - CITY OF SANTA MARIA

Associated Transportation Engineers (ATE) has prepared the following traffic, circulation and VMT study for the Sears Building Remodel Project, located in the City of Santa Maria.

We appreciate the opportunity to assist you with the project.

Associated Transportation Engineers

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Scott A. Schell Principal Transportation Planner

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## INTRODUCTION

The following report contains an analysis of the potential traffic and circulation effects of the Sears Building Remodel Project (the "Project"), proposed in the City of Santa Maria. The study evaluates the existing and future traffic conditions in the study area in order to determine the Project's consistency with the City's transportation policies. The intersections analyzed in the study were determined based on input provided by City of Santa Maria staff. An analysis of site access, circulation and queuing is provided. The study also evaluates the Project's potential CEQA transportation impacts based on the City's adopted "Vehicle Miles Traveled" (VMT) impact criteria.

## **PROJECT DESCRIPTION**

The Project site is located at the south side of Main Street east of Broadway and west of Miller Street in the City of Santa Maria, as shown on Figure 1. Figure 2 presents the Project site plan. The Project site is currently occupied by the vacant Sears Building. The Project is proposing to redevelop the building with a 50,989 SF grocery store on the first floor, a 27,242 SF apparel store on the second floor, and an additional 23,651 SF apparel store on the second floor. Parking for the Project would be provided within the adjacent existing surface parking lot and parking structures. Access to the Project is provided via the existing driveways on Broadway and Miller Street; and at the signalized Town Center Drive intersection on Main Street.

## TRAFFIC ANALYSIS SCENARIOS

The following scenarios are included in the traffic analysis.

*Existing Conditions:* This scenario describes the existing street network and evaluates peak hour operations at the key study-area intersections identified for analyses.

*Existing* + *Project:* This scenario evaluates traffic operations assuming Existing + Project traffic forecasts. The Project's consistency with the City's transportation policies is evaluated for this scenario.

*Cumulative Conditions:* This scenario evaluates traffic operations assuming the additional traffic that will be generated by approved and pending developments located in the adjacent areas of the City. Traffic volumes generated by the approved and pending projects are layered onto the Existing baseline traffic forecasts for analyses.

*Cumulative* + *Project:* This scenario evaluates operations assuming the Cumulative conditions plus the traffic generated by the Project. The Project's consistency with the City's transportation policies is evaluated for this scenario.

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Associated Transportation Engineers April 19, 2024

## **EXISTING CONDITIONS**

## **Existing Street Network**

The Project site is served by a network of highways, arterials, and collector streets, as shown on Figure 3. The following text provides a brief discussion of the major components of the study-area street network.

<u>US 101</u>, located east of the Project site, is a freeway that serves as the major north-south link through the Santa Maria Valley and is the principal inter-city route along the Pacific Coast. US 101 is a 6-lane freeway within the Santa Maria area, with 4 lanes provided north and south of the City. Access to the Project site from US 101 is provided via the Main Street interchange.

<u>Broadway (State Route 135)</u>, located west of the Project site, is a Primary Arterial roadway that extends from US 101 on the north end of the City to its junction with State Route 1 south of the Orcutt community. Broadway is a four- to six-lane arterial that serves as the primary north-south route through the Santa Maria/Orcutt area. The roadway is named "Broadway" north of Santa Maria Way and "Orcutt Expressway" south of Santa Maria Way. Broadway provides access to the Project via two existing driveway connections located west of the Project. Class II (on-street) bike lanes are provided on Broadway north and south of Main Street.

<u>Main Street (State Route 166)</u>, located north of the Project site, is a Primary Arterial roadway that extends west from US 101 as State Route (SR) 166 to the City of Guadalupe. East of US 101, Main Street extends to Stowell Road where it transitions to Philbric Road. Main Street provides access to the Project at the signalized Town Center Drive intersection. No bike facilities are provided on Main Street within the Project study-area.

<u>Miller Street</u>, located east of the site, is a north-south Secondary Arterial providing a secondary north-south access route for the eastern area of Santa Maria. Many local drivers use this facility as an alternative to Broadway for north-south travel. Within the study-area, Miller Street contains four lanes and is controlled by traffic signals at the Main Street and Cook Street intersections. Miller Street provides access at the Church Street intersection. Class II bike lanes are provided on Miller Street south of Cook Street.

<u>Cook Street</u>, located south of the Project site, is a 4-lane Collector street west of Miller Street and a 2-lane Collector street east of Miller Street. No bike facilities are provided on Cook Street within the Project study-area.

Town Center Drive, located north of the Project site, is a two-lane internal street that provides access to the Project and the Santa Maria Town Center via the connections to Main Street and Broadway. No bike facilities are provided on Town Center Drive within the Project study-area.



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<u>Church Street</u>, located east of the site, is a two-lane local street located opposite the Project access connection on Miller Street. Church Street extends east from Blosser Road to College Drive. Church Street is controlled by stop-signs at the Miller Street intersection. No bike facilities are provided on Church Street within the Project study-area.

<u>Elizabeth Street</u>, located east of the site, is a two-lane local street. No bike facilities are provided on Elizabeth Street within the Project study-area.

## Existing Pedestrian Facilities

Within the Project study area, sidewalks are currently provided on Broadway, Main Street Miller Street, and Town Center Drive. ADA accessible crosswalks with pedestrian signals heads are provided on all legs of the Main Street/Broadway, Main Street/Town Center Drive and Main Street/Miller Street intersections.

## **Existing Transit Facilities**

Transit service in the City of Santa Maria is provided by the Santa Maria Regional Transit (SMRT) service. The Santa Maria Transit Center is located on Miller Street at Boone Street (approximately 3,000 feet south of the site). A major bus stop is provided adjacent to the project site on Main street. Another transit stop is also provided on the west side of Broadway. SMRT Routes 1, 2, 3, 4, 5, 9, 11, and 12x all provide service to one or more of these transit stops, thus the Project site is well served by transit. Breeze Route 100 is a weekday bus service between the Santa Maria and Lompoc Transit Centers with seven trips per day in each direction. The closest stops to the Project site are at the Santa Maria Transit Center.

## **Intersection Operations**

Because traffic flow on urban arterials is most constrained at intersections, detailed traffic flow analyses focus on the operating conditions of critical intersections during peak travel periods. "Levels of Service" (LOS) A through F are used to rate intersection operations, with LOS A indicating very good operation and LOS F indicating poor operation (more complete definitions are contained in the Technical Appendix for reference). The City of Santa Maria considers LOS D as the performance standard for intersections (maintain LOS D or better).

The existing traffic controls and lane geometry for the study-area intersections are presented on Figure 3. Existing intersection traffic volumes were obtained from traffic count data collected in January of 2023 and March of 2024 (see Technical Appendix for count data). Counts were conducted during the AM peak commuter period (7:00-9:00 AM) and PM peak commuter period (4:00-6:00 PM). The peak 1-hour volumes were then identified for the analysis. Figure 4 presents the existing peak hour traffic volumes for the study-area intersections.

Levels of service for the signalized intersections were calculated using the intersection capacity utilization (ICU) methodology. Table 1 lists the existing traffic control and levels of service for the study-area intersections identified for the analysis.



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		AM Peak Hour		PM Peak Hour	
		ICU or		ICU or	
Intersection	Control	Delay	LOS	Delay	LOS
Broadway/Main Street (a)	Signal	0.54	LOS A	0.72	LOS C
Town Center Drive/Main Street	Signal	0.33	LOS A	0.44	LOS A
Miller Street/Main Street (a)	Signal	0.53	LOS A	0.68	LOS B
Elizabeth Street/Main Street (a)(b)	Stop-Sign	14.0 sec.	LOS B	19.3 sec.	LOS C
Miller Street/Church Street (b)	Stop-Sign	9.9 sec.	LOS A	14.7 sec.	LOS B
Broadway/Cook Street (a)	Signal	0.46	LOS A	0.64	LOS B
Miller Street/Cook Street	Signal	0.49	LOS A	0.69	LOS B

#### Table 1 Existing Levels of Service

(a) Intersection is under the jurisdiction of Caltrans.

(b) Unsignalized intersection. LOS based on average weighted control delay per vehicle in seconds.

The data presented in Table 1 show that the study-area intersections currently operate in the LOS A-C range during the AM and PM peak hours, which meet the City's LOS D operating standard.

## TRAFFIC POLICY STANDARDS

The City of Santa Maria Circulation Element considers LOS D acceptable for roadway and intersection operations, with improvements required for LOS E and F. It is noted that four of the study-area intersections are under Caltrans' jurisdiction. The current Caltrans traffic analysis guidelines are based on VMT and not LOS, thus the VMT section of this report addresses the Caltrans requirements.

## EXISTING + PROJECT CONDITIONS

## Project Trip Generation

Trip generation estimates were calculated for the Project using rates presented in the Institute of Transportation Engineers (ITE) Trip Generation Manual (11<sup>th</sup> Edition).<sup>1</sup> The rates for Supermarket (Land Use Code #850) and Apparel Store (Land Use Code #876) were used for the analysis of the proposed Project. The rates for Shopping Center (Land Use Code #820) were used for the analysis of the existing Sears building. The trip generation estimates for the existing Sears building were used as credits for the Proposed Project.

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Trip Generation, Institute of Transportation Engineers, 11th Edition, 2021.

## **Commercial Pass-By/Primary Trip Estimates**

Pursuant to ITE recommendations, the trip generation analysis also accounts for "Pass-By" trips and "Primary" trips that would be generated by the commercial uses. Pass-By trips are trips that would come from the existing traffic streams on Broadway, Miller Street, and Main Street; and would not affect the study-area street network beyond the Project site. Primary trips are trips with the sole purpose of patronizing the commercial center (i.e., from home to the store and then return home). Based on the data presented in the ITE Trip Generation manual, the Pass-By Trip percentage is approximately 24% for the grocery store and 19% for the apparel store and Sears building. Table 2 presents the net trip generation estimates (detailed worksheets contained in the Technical Appendix) for the Project with the pass-by factors.

	Pass-By	A	DT	AM Peak Hour		PN	A Peak Hour	
Size	Factor	Rate	Trips	Rate	Trips	Rate	Trips	
Proposed								
50,989 SF	0.76	93.84	3,637	2.86	111 (65/46)	8.95	347 (173/174)	
27,242 SF	0.81	66.40	1,465	1.00	22 (18/4)	4.12	91 (46/45)	
23,651 SF	0.81	66.40	1,272	1.00	19 (15/4)	4.12	78 (40/38)	
			6,374		152 (98/54)		516 (259/257)	
101,882 SF	0.81	37.01	3,055		70 (43/27)		280 (134/146)	
Net Total Trip Generation:							236 (125/111)	
	Size 50,989 SF 27,242 SF 23,651 SF 101,882 SF al Trip Genera	Pass-By           Size         Factor           50,989 SF         0.76           27,242 SF         0.81           23,651 SF         0.81           101,882 SF         0.81	Pass-By         AI           Size         Factor         Rate           50,989 SF         0.76         93.84           27,242 SF         0.81         66.40           23,651 SF         0.81         66.40           101,882 SF         0.81         37.01	Pass-By         ADT           Size         Factor         Rate         Trips           50,989 SF         0.76         93.84         3,637           27,242 SF         0.81         66.40         1,465           23,651 SF         0.81         66.40         1,272           101,882 SF         0.81         37.01         3,055           al Trip Generation:	Pass-By Factor         ADT         AM           Size         Factor         Rate         Trips         Rate           50,989 SF         0.76         93.84         3,637         2.86           27,242 SF         0.81         66.40         1,465         1.00           23,651 SF         0.81         66.40         1,272         1.00           101,882 SF         0.81         37.01         3,055	Pass-By         ADT         AM Peak Hour           Size         Factor         Rate         Trips         Rate         Trips           50,989 SF         0.76         93.84         3,637         2.86         111 (65/46)           27,242 SF         0.81         66.40         1,465         1.00         22 (18/4)           23,651 SF         0.81         66.40         1,272         1.00         19 (15/4)           Ion (1,882 SF         0.81         37.01         3,055         70 (43/27)           al Trip Generation:         3,319         82 (55/27)	Pass-By         ADT         AM Peak Hour         PM           Size         Factor         Rate         Trips         Rate         Trips         Rate         Trips         Rate           50,989 SF         0.76         93.84         3,637         2.86         111 (65/46)         8.95           27,242 SF         0.81         66.40         1,465         1.00         22 (18/4)         4.12           23,651 SF         0.81         66.40         1,272         1.00         19 (15/4)         4.12           101,882 SF         0.81         37.01 <b>3,055 70 (43/27)</b> 4.12	

Table 2Project Trip Generation – Net New Trips

(a) Trip generation based on ITE rates for Supermarket (ITE #850).

(b) Trip generation based on ITE rates for Apparel Store (ITE #876).

(c) Trip generation based on ITE rates for Shopping Center (ITE #820).

The data presented in Table 2 indicate that the Project is forecast to generate 3,319 ADT, 82 AM peak hour trips and 236 PM peak hour trips (after the credits and pass-by adjustments).

## **Project Trip Distribution**

The trip distribution pattern for the Project was developed based on existing traffic patterns observed in the study-area, consideration of the land uses in the surrounding area, and the proposed access and parking system. Given that the traffic generated by the grocery store would have a more local orientation than the traffic generated by the apparel stores and the Sears building, two separate distribution patterns were developed for these Project components. Table 3 presents the trip distribution patterns developed for the Project and Figures 5 illustrates the trip distribution and assignment of Project traffic.

		Grocery Store	Apparel & Sears
<b>Origin/Destination</b>	Direction	Percentage	Percentage
Duesekuuru	North	25%	15%
Broadway	South	30%	25%
A Aillan Chua at	North	3%	3%
Miller Street	South	10%	10%
Elizabeth Street	North	2%	2%
Main Streat	East	10%	25%
Main Street	West	10%	10%
Church Street	East	2%	2%
Cool Streat	East	3%	3%
Cook Street	West	5%	5%
Totals		100%	100%

## Table 3 Project Trip Distribution

## **Existing + Project Intersection Operations**

Levels of service were calculated for the study-area intersections assuming the Existing + Project traffic volumes shown on Figure 6. Tables 4 and 5 compare the Existing and Existing + Project level of service forecasts and identify the Project's consistency with the City's LOS D standard.



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	Existing		Existing +		
Intersection	ICU or Delay	LOS	ICU or Delay	LOS	Consistent?
Broadway/Main Street	0.54	LOS A	0.55	LOS A	Yes
Town Center Drive/Main Street	0.33	LOS A	0.36	LOS A	Yes
Miller Street/Main Street	0.53	LOS A	0.54	LOS A	Yes
Elizabeth Street/Main Street (a)	14.0 sec.	LOS B	14.1 sec.	LOS B	Yes
Miller Street/Church Street (a)	9.9 sec.	LOS A	10.7 sec.	LOS B	Yes
Broadway/Cook Street	0.46	LOS A	0.46	LOS A	Yes
Miller Street/Cook Street	0.49	LOS A	0.49	LOS A	Yes

Table 4Existing + Project Levels of Service – AM Peak Hour

(a) Unsignalized intersection. LOS based on average weighted control delay per vehicle in seconds.

Table 5Existing + Project Levels of Service – PM Peak Hour

	Existing		Existing + Project		
Intersection	ICU or Delay	LOS	ICU or Delay	LOS	Consistent?
Broadway/Main Street	0.72	LOS C	0.80	LOS C	Yes
Town Center Drive/Main Street	0.44	LOS A	0.55	LOS A	Yes
Miller Street/Main Street	0.68	LOS B	0.69	LOS B	Yes
Elizabeth Street/Main Street (a)	19.3 sec.	LOS C	20.4 sec.	LOS C	Yes
Miller Street/Church Street (a)	14.7 sec.	LOS B	19.6 sec.	LOS C	Yes
Broadway/Cook Street	0.64	LOS B	0.67	LOS B	Yes
Miller Street/Cook Street	0.69	LOS B	0.71	LOS C	Yes

(a) Unsignalized intersection. LOS based on average weighted control delay per vehicle in seconds.

The data presented in Tables 4 and 5 show that the study-area intersections are forecast to operate in the LOS A-C range during the AM and PM peak hours with Existing + Project traffic, which meet the City's LOS D operating standard.

#### CUMULATIVE ANALYSIS

#### **Cumulative Traffic Volumes**

Cumulative traffic volumes were forecast for the study-area intersections assuming development of the approved and pending projects located in the adjacent portions of the City (list of cumulative projects is contained in the Technical Appendix). Trip generation estimates were developed for the cumulative projects using ITE rates or from traffic studies prepared for the cumulative projects (cumulative trip generation calculations are contained in the Technical Appendix). Traffic generated by the cumulative projects was then added to the Existing volumes to produce the Cumulative traffic forecasts. Figure 7 shows the Cumulative traffic volumes and Figure 8 shows the Cumulative + Project volumes.

#### **Cumulative Intersection Operations**

Levels of service were calculated for the study-area intersections assuming the Cumulative and Cumulative + Project traffic volumes presented on Figures 7 and 8. Tables 6 and 7 compare the Cumulative and Cumulative + Project levels of service forecasts and identify the Project's consistency with the City's LOS D standard.

	Cumulative		Cumulative		
Intersection	ICU or Delay	LOS	ICU or Delay	LOS	Consistent?
Broadway/Main Street	0.58	LOS A	0.60	LOS A	Yes
Town Center Drive/Main Street	0.37	LOS A	0.40	LOS A	Yes
Miller Street/Main Street	0.57	LOS A	0.58	LOS A	Yes
Elizabeth Street/Main Street (a)	14.9 sec.	LOS B	15.1 sec.	LOS C	Yes
Miller Street/Church Street (a)	10.1 sec.	LOS B	11.0 sec.	LOS B	Yes
Broadway/Cook Street	0.48	LOS A	0.49	LOS A	Yes
Miller Street/Cook Street	0.51	LOS A	0.52	LOS A	Yes

Table 6Cumulative + Project Levels of Service – AM Peak Hour

(a) Unsignalized intersection. LOS based on average weighted control delay per vehicle in seconds.



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	Cumula	ative	Cumulative -		
Intersection	ICU or Delay	LOS	ICU or Delay	LOS	Consistent?
Broadway/Main Street	0.78	LOS C	0.85	LOS D	Yes
Town Center Drive/Main Street	0.47	LOS A	0.58	LOS A	Yes
Miller Street/Main Street	0.70	LOS B	0.72	LOS C	Yes
Elizabeth Street/Main Street (a)	21.1 sec.	LOS C	22.4 sec.	LOS C	Yes
Miller Street/Church Street (a)	15.5 sec.	LOS C	21.4 sec.	LOS C	Yes
Broadway/Cook Street	0.68	LOS B	0.71	LOS C	Yes
Miller Street/Cook Street	0.72	LOS C	0.74	LOS C	Yes

Table 7Cumulative + Project Levels of Service - PM Peak Hour

(a) Unsignalized intersection. LOS based on average weighted control delay per vehicle in seconds.

As shown in Tables 6 and 7, the study-area intersections are forecast to operate in the LOS A-D range during the AM and PM peak hours with Cumulative and Cumulative + Project traffic, which meet the City's LOS D standard.

## SITE ACCESS AND CIRCULATION

## **Access Driveways**

As noted in the Project Description, parking for the Project would be provided within the existing Town Center Mall surface parking lot and parking structures. Vehicular access would be provided via the existing driveways on Broadway and Miller Street; and at the signalized Town Center Drive intersection on Main Street. The parking structure entrance on Main Street at Town Center Drive is signalized and is forecast to operate at LOS A with Cumulative + Project volumes, indicating good operations with the addition of Project traffic. Additionally, the driveway at the unsignalized Miller Street/Church Street intersection is forecast to operate in the LOS B - C range with Cumulative + Project volumes, further indicating good operations with the addition of Project traffic. The Project also has a standard driveway entrance on Broadway with right-turn in and out access.

#### Main Street Queuing Analysis

A queuing analysis was completed for the Main Street intersections adjacent to the Project site to determine if future vehicle queues will be accommodated in the available storage. The analysis reviews queue forecasts for the left-turn lanes under Cumulative + Project scenarios.

The queueing analysis was completed using the SYNCHRO software program. The SYNCHRO software implements the Highway Capacity Manual (HCM) operations methodology and predicts both "50th Percentile" and "95th Percentile" queue forecasts for the peak period. The 50th percentile queue forecasts represent the average queues during the peak period and are recommended for design purposes. Worksheets showing the queue forecasts are contained in the Technical Appendix. Tables 8 and 9 summarize the lane storage provided and the average (50<sup>th</sup>) and peak (95<sup>th</sup>) queue forecasts for Main Street intersections adjacent to the site.

Table 8
Cumulative + Project AM Peak Hour - Left-Turn Storage and Queues

		Cumulat		
Intersection	Storage Length	50 <sup>th</sup> % Queue	95 <sup>th</sup> % Queue	Exceeds Storage?
Main/Broadway				
<ul> <li>WB Left-Turn #1</li> </ul>	450 Feet	143 Feet	178 Feet	No
WB Left-Turn #2	450 Feet	133 Feet	184 Feet	No
<ul> <li>SB Left-Turn</li> </ul>	520 Feet	175 Feet	282 Feet	No
Main St/Town Center Drive				
<ul> <li>WB Left-Turn</li> </ul>	240 Feet	17 Feet	40 Feet	No
<ul> <li>EB Left-Turn</li> </ul>	140 Feet	< 1 Vehicle	< 1 Vehicle	No
<ul> <li>NB Left-Turn</li> </ul>	130 Feet	38 Feet	66 Feet	No
Main St/Miller St				
<ul> <li>WB Left-Turn</li> </ul>	230 Feet	95 Feet	126 Feet	No
EB Left-Turn	250 Feet	37 Feet	78 Feet	No

		Cumulati	Cumulative + Project							
Intersection	Storage Length	50 <sup>th</sup> % Queue	95 <sup>th</sup> % Queue	Exceeds Storage?						
Main/Broadway										
WB Left-Turn #1	450 Feet	120 Feet	180 Feet	No						
WB Left-Turn #2	450 Feet	124 Feet	174 Feet	No						
SB Left-Turn	520 Feet	179 Feet	276 Feet	No						
Main St/Town Center Drive										
WB Left-Turn	240 Feet	98 Feet	140 Feet	No						
EB Left-Turn	140 Feet	18 Feet	33 Feet	No						
NB Left-Turn	130 Feet	82 Feet	120 Feet	No						
Main St/Miller St										
WB Left-Turn	230 Feet	135 Feet	208 Feet	No						
EB Left-Turn	250 Feet	77 Feet	146 Feet	No						

 Table 9

 Cumulative + Project PM Peak Hour - Left-Turn Storage and Queues

The data presented in Tables 8 and 9 indicate that all of the storage lengths at the intersections satisfy the  $50^{th}$  and  $95^{th}$  percentile queue forecasts with the Cumulative + Project traffic volume forecasts.

## ACCIDENT ANALYSIS

An accident analysis was completed to evaluate the accident rates at the Miller Street/Church Street and Main Street/Elizabeth Street intersections which are unsignalized. Accident data was obtained from the City of Santa Maria for the most current 3-year period of accident records.

It is important to note that accident data is used as a screening tool to identify potential safety problems. The rate of accidents was calculated for each intersection and then compared to California statewide averages for similar facilities to identify potential safety issues. By nature, accident rates experienced on a facility are often higher than the statewide average rate for similar facilities since the statewide averages are comprised of lower-than-average rates + higher-than-average rates (lower + higher = average).

If the accident rate experienced on a facility is higher than the statewide average, the Caltrans significance test is performed to determine if the number of accidents that occurred on the facility is statistically significant. If the number of accidents experienced is statistically significant, more detailed safety investigations are performed to determine if there are accident patterns that can be corrected by changing design features of the facility (e.g., widen traffic lanes, widen roadway shoulders, change roadway curvatures, add signs, install traffic signals, etc.).

Accident rates were calculated for the two intersections adjacent to the Project site using the 3-year accident history. The "area of influence" for each intersection is defined as within 250 feet of the intersection. The rate of accidents was calculated and then compared to California statewide average for similar facilities. Table 10 lists the actual rate of accidents for the 3-year period and compares the rates to the California statewide averages for similar intersections (see accident rate calculations contained in the Technical Appendix for more details).

Location	# Accidents	Accident Rate(a)	Statewide Average Rate(a)
Main Street/Elizabeth Street	4 Accidents	0.16 per mev	0.36 per mev
Miller Street/Church Street	4 Accidents	0.24 per mev	0.36 per mev
			•

Table 10
<b>Project Intersections - Accident Rates</b>

(a) Accident rates per million entering vehicles (mev).

<u>Main Street/Elizabeth Street</u>. As shown in Table 10, there were 4 accidents reported at this intersection within the 3-year period. The rate of accidents was 0.16 accidents per million entering vehicles and the California statewide average for similar intersections is 0.36 accidents per million entering vehicles. Thus, the rate of accidents is well below the statewide average and further investigation is not required.

<u>Miller Street/Church Street</u>. As shown in Table 10, there were 4 accidents reported at this intersection within the 3-year period. The rate of accidents was 0.24 accidents per million entering vehicles and the California statewide average for similar intersections is 0.36 accidents per million entering vehicles. Thus, the rate of accidents is below the statewide average and further investigation is not required.

#### VMT ANALYSIS

Per the State's Natural Resource Agency Updated Guidelines for the Implementation of the CEQA adopted in 2018, Vehicle Miles Traveled (VMT) has been designated as the most appropriate measure of transportation impacts. "Vehicle Miles Traveled" refers to the amount and distance of automobile travel attributable to a project. Other relevant considerations may include the effects of the project on transit and non-motorized travel.

For land use projects, vehicle miles traveled exceeding an applicable threshold of significance may indicate a significant impact. The Governor's Office of Planning and Research (OPR) published a Technical Advisory on Transportation that includes recommendations regarding assessment of VMT, thresholds of significance, and mitigation measures.<sup>2</sup> The Technical Advisory provides screening tools to determine when a project may have a significant VMT impacts.



<sup>&</sup>lt;sup>2</sup> <u>Technical Advisory on Evaluating Transportation Impacts in CEQA</u>, Governor's Office of Planning and Research, December 2018.

## VMT Thresholds Analysis

The OPR technical advisory provides the following guidance for evaluating redevelopment projects:

## Redevelopment Projects

Where a project replaces existing VMT-generating land uses, if the replacement leads to a net overall decrease in VMT, the project would lead to a less-than-significant transportation impact. If the project leads to a net overall increase in VMT, then the thresholds described above should apply.

## City Thresholds

The City of Santa Maria's adopted Environmental Procedures and Guidelines manual ("CEQA Guidelines")<sup>3</sup> contain thresholds and methodologies for assessing potential VMT impacts for Projects located in the City, which are reviewed below. Consistent with the recommendations in the OPR Technical Advisory, Section 4.3.1 of the City of Santa Maria's CEQA Guidelines establishes thresholds of significance for redevelopment projects, as follows:

Pursuant to guidance set forth in CEQA Guidelines Section 15064.3, for retail development projects, redevelopment projects, medical development projects, and infrastructure projects that require a VMT analysis the City has adopted "net change" in VMT as the applicable threshold for determining a significant impact (i.e., if the with-project VMT is greater than the without-project VMT).

As noted previously, the Project is proposing to redevelop the existing Sears building, which was a regional based retail facility, into a grocery store and apparel stores, which are more locally oriented retail uses. Furthermore, the Project is not proposing to increase the size of the building area. The existing Sears building attracted customers from the entire City and the community of Orcutt to the south, as well as the San Luis Obispo County areas located directly north of the City. The proposed grocery store would provide convenient shopping opportunities for the existing and future residential neighborhoods located adjacent to the Project site. The nearest major grocery store is the Vons Supermarket which is approximately 0.5 miles to the east at the Main Street/College Drive intersection. Approximately 0.6 miles to the west is the La Favorita Market. To the south, approximately 1.2 miles, are the Foods Co and the Vallarta Supermarkets. Given the change of use from a regional destination to a local oriented destination, it is anticipated that the proposed Project, consisting of predominantly grocery store trips, will result in a significant reduction in overall VMT within the City compared to the existing Sears building.

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<sup>&</sup>lt;sup>3</sup> <u>City of Santa Maria Environmental Procedures and Guidelines</u>, City of Santa Maria, Amended November 3, 2020.

## **REFERENCES AND PERSONS CONTACTED**

#### **Associated Transportation Engineers**

Scott A. Schell, Principal Transportation Planner Jiho Ha, Transportation Engineer II Glenn Manaois, Transportation Engineer I

#### **Persons Contacted**

Mark Mueller, PE, City of Santa Maria Luis Magallon, PE, City of Santa Maria Dana Eady, Planning Division Manager, City of Santa Maria

#### References

Highway Capacity Manual, Transportation Research Board, 7th Edition, 2022.

Trip Generation, Institute of Transportation Engineers, 11th Edition, 2021.

Technical Advisory on Evaluating Transportation Impacts in CEQA, Governor's Office of Planning and Research, December 2018.

<u>City of Santa Maria Environmental Procedures and Guidelines</u>, City of Santa Maria, Amended November 3, 2020

## **TECHNICAL APPENDIX**

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CITY OF SANTA MARIA APPROVED AND PENDING PROJECTS LIST

APPROVED AND PENDING PROJECT TRIP GENERATION WORKSHEET

MAIN STREET QUEUING ANALYSIS

ACCIDENT RATE WORKSHEETS

#### INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

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Reference 2	-	Town Center Drive/Main Street
Reference 3	-	Miller Street/Main Street
Reference 4	-	Elizabeth Street/Main Street
Reference 5	-	Miller Street/Church Street
Reference 6	-	Broadway/Cook Street
Reference 7	-	Miller Street/Cook Street

LEVEL OF SERVICE DEFINITIONS



Since 1978

Richard L. Pool, P.E. Scott A. Schell

LOS	Delay (a)	V/C Ratio	Definition
А	< 10.0	< 0.60	Progression is extremely favorable. Most vehicles arrive during the green phase. Many vehicles do not stop at all.
В	10.1 - 20.0	0.61 - 0.70	Good progression, short cycle lengths, or both. More vehicles stop than with LOS A, causing higher levels of delay.
С	20.1 - 35.0	0.71 - 0.80	Only fair progression, longer cycle lengths, or both, result in higher cycle lengths. Cycle lengths may fail to serve queued vehicles, and overflow occurs. Number of vehicles stopped is significant, though many still pass through intersection without stopping.
D	35.1 - 55.0	0.81 - 0.90	Congestion becomes more noticeable. Unfavorable progression, long cycle lengths and high v/c ratios result in longer delays. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	55.1 - 80.0	0.91 - 1.00	High delay values indicate poor progression, long cycle lengths and high v/c ratios. Individual cycle failures are frequent
F	> 80.0	> 1.00	Considered unacceptable for most drivers, this level occurs when arrival flow rates exceed the capacity of lane groups, resulting in many individual cycle failures. Poor progression and long cycle lengths may also contribute to high delay levels.

## Signalized Intersection Level of Service Definitions

(a) Average control delay per vehicle in seconds.

#### Unsignalized Intersection Level of Service Definitions

The HCM<sup>1</sup> uses *control delay* to determine the level of service at unsignalized intersections. Control delay is the difference between the travel time actually experienced at the control device and the travel time that would occur in the absence of the traffic control device. Control delay includes deceleration from free flow speed, queue move-up time, stopped delay and acceleration back to free flow speed.

LOS	Control Delay Seconds per Vehicle
А	< 10.0
В	10.1 - 15.0
С	15.1 - 25.0
D	25.1 - 35.0
E	35.1 - 50.0
F	> 50.0

<sup>&</sup>lt;sup>1</sup> Highway Capacity Manual, National Research Board, 2016.

TRAFFIC COUNT DATA

LOCATION: E CITY/STATE:	Broadv Santa	vay N Maria,	Main St CA												QC DATE:	: <b>JOB #</b> Wed,	<b>‡:</b> 1601 Jan 18	17807 2023
640 ← 81 . 455 614 → 78	836 106 59 • • • • 09 • • 101 45 837	668 4 136 6 136 6 123 6 80	133 ← 732 434 165 → 717			Pe Pea	ak-Hou k 15-M Qua Data TH	r: 7:45 in: 7:4	SAM	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$								
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7:30 AM 7:45 AM	17	86 91	18 23	1	27 39	121 184	26 29	0	18	72	12 14	0	25 39	89 121	29 28	0	541 742	1998
8:00 AM	21	118	25	0	33	167	32	0	17	107	23	1	52	105	33	3	737	2441
8:15 AM 8:30 AM	22 35	132 115	35 40	0	34 30	130 113	18 27	0	27 14	111 107	32 9	1	28 43	92 116	36 36	0	698 685	2718 2862
8:45 AM	20	82	20	0	29	122	18	0	25	88	20	1	59	127	20	0	631	2751
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		-	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles Heavy Trucks	80 4	364 12	92 0	12	156 4	736 28	116 4	0	84 8	520 40	56 4	0	156 4	484 36	112 12	0	29 15	68 66
Pedestrians Bicycles Scooters	0	16 0	0		0	24 0	0		0	56 0	0		0	4 0	0		10 0	0

Comments:

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4:30 PM	40	210	40	8	40	130	38	3	38	130	27	3	65	138	52	1	963	
4:45 PM	37	203	31	4	56	135	34	0	45	142	29	0	51	136	48	0	951	3928
5:15 PM	47	163	29	5	43	159	38	Ō	54	127	22	Ó	45	143	45	3	923	3869
5:30 PM	37	192	35	7	50	158	37	2	30	111	27	0	52	108	38	0	884	3790
5:45 PM	38	154	31	5	49	154	32	U	35	109	25	2	43	128	45	U	850	3689
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Comments:

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7:30 AM	0	0	6	0	0	1	6	0	3	140	5	0	2	158	3	0	324	
7:45 AM	3	1	5	0	3	2	14	0	4	158	0	0	7	209	8	0	414	1201
8:00 AM	0	0	3	0	3	1	2	0	2	173	0	0	1	173	4	1	363	1359
8:15 AM	1	0	3	0	1	0	3	0	3	154	2	0	2	185	3	0	357	1458
8:30 AM	0	0	6	0	2	0	4	0	5	198	1	0	3	202	5	0	426	1560
8:45 AM	1	0	1	0	1	0	7	0	3	157	0	0	8	173	5	0	356	1502
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Peak 15-Min Flowrates All Vehicles Heavy Trucks	Left 0 0	North Thru 0 0	bound Right 24 0	U 0	Left 8 0	South Thru 0 0	bound Right 16 0	U 0	Left 20 0	Eastb Thru 792 56	ound Right 4 0	U 0	Left 12 0	Westl Thru 808 36	Dound Right 20 0	U 0	To 17 9	tal 04 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses	Left 0 0	North Thru 0 0	bound Right 24 0	U 0	Left 8 0	South Thru 0 0	bound Right 16 0	U 0	Left 20 0	Eastb Thru 792 56	Right 4 0	U 0	Left 12 0	Westh Thru 808 36	Right 20 0	U 0	To 17 9	tal 04 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians	Left 0 0	North Thru 0 0 4	bound Right 24 0	0	Left 8 0	South Thru 0 0 0	bound Right 16 0	U 0	Left 20 0	Eastb Thru 792 56 0	Right 4 0	0	Left 12 0	Westh Thru 808 36 0	Dound Right 20 0	0	To 17 9	tal 04 2
Peak 15-Min Flowrates Heavy Trucks Buses Pedestrians Bicycles	Left 0 0	North Thru 0 4 0	bound Right 24 0 0	U 0	Left 8 0	South Thru 0 0 0 0	bound Right 16 0 0	U 0	Left 20 0	Eastb Thru 792 56 0 0	Right 4 0 0	U 0	Left 12 0	Westh Thru 808 36 0 0	Dound Right 20 0	U 0	To 17 9 4	tal 04 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians Bicycles Scooters	Left 0 0	North Thru 0 4 0	bound Right 24 0 0	U 0	Left 8 0	South Thru 0 0 0	bound Right 16 0 0	U 0	Left 20 0	Eastb Thru 792 56 0 0	Right 4 0 0	U 0	Left 12 0 0	Westl Thru 808 36 0 0	Dound       Right       20       0	U 0	To 17 9 4	tal 04 2

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Period		(North	bound)			(South	bound)			(Eastb	ound)			(West	bound)		Total	Hourly
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4:00 PM	2	0	13	0	3	0	10	0	9	223	3	0	5	245	12	0	525	
4:15 PM	3	2	10	0	2	0	13	0	7	258	4	2	4	251	10	0	566	
4:30 PM	3	0	6	0	5	0	13	0	7	217	2	0	4	231	15	0	503	
4:45 PM	2	1	17	0	2	0	11	0	6	286	0	0	4	213	7	0	549	2143
5:00 PM	1	0	10	0	1	2	14	0	7	287	1	0	1	259	2	0	585	2203
5:15 PM	4	0	13	0	5	0	13	0	5	261	3	0	8	236	6	0	554	2191
5:30 PM	7	0	9	0	3	0	15	0	4	211	5	0	6	173	4	0	437	2125
5:45 PM	3	2	7	0	1	1	6	0	5	182	0	0	5	208	6	0	426	2002
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	ound		Te	tal
Peak 15-Min Flowrates	Left	North Thru	bound Right	U	Left	South Thru	bound Right	U	Left	Eastb Thru	ound Right	U	Left	Westb Thru	oound Right	U	To	tal
Peak 15-Min Flowrates All Vehicles	Left 4	North Thru 0	bound Right 40	U 0	Left 4	South Thru 8	bound Right 56	U	Left 28	Eastb Thru 1148	ound Right 4	U 0	Left 4	Westa Thru 1036	oound Right 8	U 0	To 23	tal 40
Peak 15-Min Flowrates All Vehicles Heavy Trucks	Left 4 0	North Thru 0 0	bound Right 40 0	U 0	Left 4 0	South Thru 8 0	bound Right 56 4	U O	Left 28 0	Eastb Thru 1148 52	Right 0	U 0	Left 4 0	Westb Thru 1036 16	Right 8 0	U 0	To 23 7	tal 40 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses	Left 4 0	North Thru 0 0	bound Right 40 0	U 0	Left 4 0	South Thru 8 0	bound Right 56 4	U 0	Left 28 0	Eastb Thru 1148 52	Right 4 0	U 0	Left 4 0	Westb Thru 1036 16	Right 8 0	U 0	To: 23 7	tal 40 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians	Left 4 0	North Thru 0 0 12	bound Right 40 0	U 0	Left 4 0	South Thru 8 0 0	bound Right 56 4	U O	Left 28 0	Eastb Thru 1148 52 4	Right 0	U 0	Left 4 0	Westh Thru 1036 16 4	Right 8 0	U 0	To 23 7 2	tal 40 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians Bicycles	<b>Left</b> 4 0	North Thru 0 0 12 0	bound Right 40 0 4	U 0	Left 4 0	South Thru 8 0 0 0	bound Right 56 4 0	U 0	Left 28 0	Eastb Thru 1148 52 4 0	Right 4 0 0	U 0	<b>Left</b> 4 0	Westh Thru 1036 16 4 0	Right 8 0 4	U 0	To 23 7 2 8	tal 40 2 0
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians Bicycles Scooters	Left 4 0	North Thru 0 12 0	bound Right 40 0 4	U 0	Left 4 0	South Thru 8 0 0 0	bound Right 56 4 0	U 0	Left 28 0 0	Eastb Thru 1148 52 4 0	round Right 4 0 0	U 0	Left 4 0	Westb Thru 1036 16 4 0	bound Right 8 0 4	U 0	To 23 7 2 8	tal 40 2 0 3

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LOCATION: S Miller St -- E Church St QC JOB #: 16509101 CITY/STATE: Santa Maria, CA DATE: Thu, Mar 21 2024 Peak-Hour: 7:30 AM -- 8:30 AM 455 1.8 2.7 376 Peak 15-Min: 7:45 AM -- 8:00 AM ↓
32 421 2 **↓** 0 **↑** 0 1.9 ŧ ŧ 53 🔶 **L** 23 **+** 38 0 **+** 0 **, 4**.3 **+** 2.6 7 ٠ 0.86 0 🔸 **+** 0 0 🌩 **+** 1 0 🔹 0 🥆 12 🔸 5 🥆 € 14 
→ 56 • 0 € **↑** 2.6 **r** 1.9 ŧ ŧ ٠ 464 ΔΔΔ 1.7 23 TRUE DATA TO IMPROVE MOBILITY 0 3 0 J 1 0 🖌 **t** 0 570 9 3 1 🔺 **4** 3 ¢ 0 7 **F** 0 • 0 ŧ 0 4 N/A N/A \_**\_** ÷ • • t t N/A → N/A N/A ⇒ ← N/A 0 1 1 Þ ç ٦ 1 ŧ h ŧ C N/A N/A ٠ S Miller St S Miller St F Church St E Church Ct

Period		(North	bound)			(South	bound)			(Eastb	ound)			(West	bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:00 AM	0	40	1	0	1	43	6	0	0	3	1	0	4	1	5	0	105	
7:15 AM	2	37	3	1	1	54	4	0	0	0	1	0	1	0	6	0	110	
7:30 AM	2	75	5	8	1	97	8	0	3	0	1	0	4	1	6	0	211	
7:45 AM	9	93	31	10	1	117	7	0	1	0	2	0	2	0	4	0	277	703
8:00 AM	6	82	8	6	0	110	10	0	1	0	1	0	7	0	6	0	237	835
8:15 AM	3	96	10	0	0	97	7	0	2	0	1	0	1	0	7	0	224	949
8:30 AM	1	76	3	0	0	84	9	0	0	0	0	0	1	1	3	0	178	916
8:45 AM	3	75	3	0	1	81	6	0	2	0	0	0	1	0	2	0	174	813
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		То	tal
Peak 15-Min Flowrates	Left	North Thru	bound Right	U	Left	South Thru	bound Right	U	Left	Eastb Thru	ound Right	U	Left	Westl Thru	bound Right	U	То	tal
Peak 15-Min Flowrates	Left 36	North Thru 372	bound Right 124	U 40	Left 4	South Thru 468	bound Right 28	U	Left 4	Eastb Thru 0	ound Right 8	U 0	Left 8	Westl Thru 0	Dound Right 16	U 0	To 11	tal .08
Peak 15-Min Flowrates All Vehicles Heavy Trucks	Left 36 0	North Thru 372 0	bound Right 124 4	U 40	Left 4 0	South Thru 468 16	bound Right 28 0	U 0	Left 4 0	Eastb Thru 0 0	Right 8 0	U 0	Left 8 0	Westl Thru 0 0	Dound Right 16 4	U 0	To 11 2	tal .08 .4
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses	Left 36 0	North Thru 372 0	bound Right 124 4	U 40	Left 4 0	South Thru 468 16	bound Right 28 0	U 0	Left 4 0	Eastb Thru 0 0	oound Right 8 0	U 0	Left 8 0	Westl Thru 0 0	oound Right 16 4	U 0	To 11 2	tal .08 .4
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians	Left 36 0	North Thru 372 0 12	bound Right 124 4	U 40	Left 4 0	South Thru 468 16 0	bound Right 28 0	U 0	Left 4 0	Eastb Thru 0 0 8	Right 8 0	U 0	Left 8 0	Westl Thru 0 0 4	night Right 16 4	0	To 11 2 2	tal 08 4
Peak 15-Min Flowrates Heavy Trucks Buses Pedestrians Bicycles	Left 36 0	North Thru 372 0 12 8	bound Right 124 4 0	U 40	Left 4 0	South Thru 468 16 0 12	bound Right 28 0 0	U 0	Left 4 0	Eastb Thru 0 0 8 0	Right 8 0 0	U 0	Left 8 0	Westl Thru 0 4 0	Dound Right 16 4 0	U 0	To 11 2 2 2	tal .08 .4 .4 .0
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians Bicycles Scooters	Left 36 0	North Thru 372 0 12 8	bound Right 124 4 0	U 40	Left 4 0	South Thru 468 16 0 12	bound Right 28 0 0	U 0	Left 4 0	Eastb Thru 0 0 8 0	Right 8 0 0	<b>U</b> 0	Left 8 0	Westl Thru 0 0 4 0	Dound Right 16 4 0	U 0	To 111 2 2 2	tal 08 4 4 0

Report generated on 4/1/2024 4:07 PM

#### LOCATION: S Miller St -- E Church St QC JOB #: 16509102 CITY/STATE: Santa Maria, CA DATE: Thu, Mar 21 2024 Peak-Hour: 4:30 PM -- 5:30 PM 561 2.3 1.3 818 Peak 15-Min: 5:15 PM -- 5:30 PM ♦ ▲ 105 455 1 **↓** 0 **↑** 0 2.9 ÷ . 143 🔶 34 🌶 **t** 51 0 **+** 0 **+** 2 **+** 1.5 **•** 66 0.93 0 🔸 0 11 🔺 ٠ 6 4 **€** 9 **→** 44 74 🔶 29 🍾 0 🔸 0 🦡 **°** 0 **↑** 1.4 ۲ 3.1 ŧ ŧ ŧ ٠ 495 799 2.6 1.4 TRUE DATA TO IMPROVE MOBILITY 0 2 0 ÷ J 1 4 **J t** 0 AD 9 2 0 🔸 **+** 0 0 7 **f** 0 ŧ ۴ • 0 0 1 N/A N/A \_**\_** ÷ • 1 t t N/A → N/A ⇒ ← N/A N/A 1 Þ a Î ç r ħ ŧ c 4 N/A N/A ٠ S Miller St S Miller St E Church St E Church St 15-Min Count Period Beginning At Hourly Totals (Northbound) (Southbound) (Eastbound) (Westbound) Total Loft

208	ιεπ	Inru	Right	U	Left	Ihru	Right	U	Left	Ihru	Right	U	Left	Ihru	Right	U		
4:00 PM	15	205	6	0	0	102	16	0	6	5	8	0	1	3	15	0	382	
4:15 PM	6	171	8	1	0	105	16	0	4	1	4	0	3	1	12	0	332	
4:30 PM	6	163	8	0	0	119	23	0	6	3	8	0	2	1	8	0	347	
4:45 PM	11	170	8	0	0	103	26	0	9	4	6	0	3	3	14	0	357	1418
5:00 PM	5	197	8	0	1	112	29	0	10	2	8	0	3	2	15	0	392	1428
5:15 PM	10	203	8	2	0	121	27	0	9	2	7	0	1	0	14	0	404	1500
5:30 PM	8	152	8	3	0	118	24	0	6	0	6	0	2	3	8	0	338	1491
5:45 PM	7	134	4	0	0	108	19	0	6	2	4	0	2	2	6	0	294	1428
		North	hound			South	hound			Eacth	aund			Worth	ound			
Peak 15-Min		NOTUI	bound			Journ	bound			EdSIL	ouna			west	Journa		Та	hal
Peak 15-Min Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Tot	tal
Peak 15-Min Flowrates All Vehicles	Left 40	Thru 812	Right 32	U 8	Left 0	Thru 484	Right	U 0	Left 36	Thru 8	Right	U 0	Left 4	Thru 0	Right 56	U 0	To:	tal 16
Peak 15-Min Flowrates All Vehicles Heavy Trucks	Left 40 0	Thru 812 8	Right 32 0	U 8	Left 0 0	Thru 484 8	Right 108 0	U 0	Left 36 0	Thru 8 0	Right 28 0	U 0	Left 4 0	Thru 0 0	Right 56 0	U 0	To: 16: 10	tal 16 6
All Vehicles Heavy Trucks Buses	Left 40 0	Thru 812 8	Right 32 0	U 8	Left 0 0	South           Thru           484           8	Right 108 0	U 0	Left 36 0	Thru 8 0	Right 28 0	U 0	Left 4 0	Thru 0 0	Right 56 0	U 0	To: 16. 10	tal 16 6
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians	Left 40 0	Thru 812 8	Right 32 0	U 8	Left 0 0	South           Thru           484           8           0	Right 108 0	U 0	Left 36 0	Thru 8 0 12	Right 28 0	0	Left 4 0	Thru 0 0 8	Right 56 0	U 0	To <sup>1</sup>	tal 16 6 0
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians Bicycles	Left 40 0	North           Thru           812           8           0           0           0	Right 32 0	U 8	Left 0 0	South           Thru           484           8           0           8	Right 108 0	U 0	Left 36 0 4	8 0 12 0	Right 28 0	0	Left 4 0	Thru 0 0 8 0	Right 56 0	U 0	To <sup>1</sup>	tal 16 6 0 2
Peak 15-Min Flowrates All Vehicles Heavy Trucks Buses Pedestrians Bicycles Scooters	Left 40 0	North           Thru           812           8           0           0           0	Right 32 0	U 8	Left 0 0	South           Thru           484           8           0           8	Right 108 0	U 0	Left 36 0 4	8 0 12 0	Right 28 0 0	U 0	Left 4 0	Thru 0 0 8 0	Right     56     0	0	To <sup>1</sup>	tal

Report generated on 4/1/2024 4:07 PM

CITT/STATE:	Santa	Maria,	CA												DATE:	Wed,	Jan 18	2023
209 ← 92 . 135 293 → 66	805 64 692 • • • • 08 • • 67 552 821	670 7 44 9 ← 2 79 698	24 ← 160 84 52 → 256			Pe Pea	ak-Hou k 15-M Qua DATA TH	r: 7:45 in: 8:1	AM	- 8:45 / 8:30	AM AM			6.2 ← 4.3 8.1 6.1 → 4.5	46 63 42 • • • • • 0 4 41	45	■ 16.7 ← 9 ■ 10.7 ■ 3.8 → 0	).4 5.6
22		• [ • ] • [	13		-	3		ļļ				-		0 0 1			• 0 • 1 • 0	
+ <i>3</i> N/A + 3	* N// • •		► N/A ►		-		<ul> <li></li> <li></li> <li></li> </ul>	•	1	↑	*	-		N/A			t ► N/A F	
15-Min Count Period		S Broa (North	idway bound)			S Broa (South	adway bound)			Coc (Eastb	ok St bound)			Coc (West	ok St bound)		Total	Hourly
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right 7	U	105	iotais
7:15 AM	6	78	3	0	7	87	7	0	12	21	8	0	6	11	5	0	251	
7:30 AM 7:45 AM	11 8	98 123	8	1	6 12	221	14 15	0	15	38	13	0	11 12	16 21	3 6	0	340 504	1290
8:00 AM	17	126	23	1	9	189	15	0	16	38	15	0	16	33	9	0	507	1602
8:30 AM	18	135	18	2	8	107	19	1	21	29	10	0	8	10	3	0	393	1903
8:45 AM	11	106	12	2	11	182	20	0	13	20	9	0	16	25	11	0	438	1890
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		Tot	tal
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles Heavy Trucks	72 0	672 32	84 4	12	52 0	720 20	60 4	4	148 8	120 4	112 0	0	64 4	64 4	24 0	0	22 8	)8 )8
Buses Pedestrians Bicycles Scooters	0	8 0	0		0	0 0	0		0	16 0	0		0	8 0	0		3: C	2

LOCATION: S CITY/STATE:	i Broad Santa	dway Maria,	Cook S CA	St											QC DATE:	C JOB i Wed,	<b>#:</b> 1601 Jan 18	17802 2023
471 ← 142 _ 198 = 439 → 99 ٦	939 100 76 939 100 76 939 100 76 939 939 939 100 76 939 939 100 76 939 100 76 939 939 100 76 939 100 76 939 100 76 100 7	1179 5 74 6 € 8 € 74 6 € 74 6 € 74 74 74 74 74 74 74 74 74 74	111 ← 507 256 140 → 316	,		Pe Pea	eak-Hou k 15-M	ur: 4:00 lin: 4:1	D PM 15 PM	- 5:00 I 4:30	PM PM		:	28 ← 3.5 3 2.7 → 1	21 2 1/ 2 1/ 2 4 2 7 4 4 7 14	2.5 4 95 4 95 8 35 16	• 72 • • • 43 • 14 • •	4.1
19		+ [     	25		-	8	: ↓ ↓	Ţ	•		<b>₽</b>	-		0 0 0			■ 1 ● 0 ■ 0	
← 3 N/A → + 7	* N/4 • *		← N/A →		-		≁ →	•	) †	↑ ſ* [	<b>*</b>	-		N/A			⊾ ► N/A	
15-Min Count Period		S Broa (North	adway bound)			S Broa (South	adway Ibound)			Coc (Eastb	ok St ound)			Coc (West	ok St bound)		Total	Hourly
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TULAIS
4:00 PM 4:15 PM	28 28	233	13 23	2	14 21	204	35 24	5	39 32	44 47	22 29	0	43 46	76 74	34 24	0	792 797	
4:30 PM	26	217	4	3	12	173	26	2	38	65	24	0	29	65	30	0	714	2020
5:00 PM	29	231	25	3	14	176	27	1	43	69	37	0	33	53	36	0	780	2968
5:15 PM 5:30 PM	22 15	215 202	19 19	0	20 11	200 168	19 32	2	23 37	44 54	16 20	0	17 25	30 50	23 17	0	650 655	2821 2762
5:45 PM	25	208	21	2	14	202	16	Ō	34	33	17	õ	20	38	16	0	646	2731
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		To	tal
riowrates	Left	Thru	Right	U	Left	Thru	Right	U 12	Left	Thru	Right	U	Left	Thru	Right	U	24	00
All Vehicles Heavy Trucks Buses Pedestrians Bicycles Scooters	0	928 16 28 0	92 4 0	8	84 12 0	848 8 112 0	96 4 0	12	128 8 0	188 8 16 0	0	U	184 4 0	296 8 20 0	96 12 4	U	31 8 17 4	88 4 /6
Comments:																		

LOCATION: S CITY/STATE:	5 Milleı Santa	r St E Maria,	Cook S CA	St											QC DATE:	: <b>JOB i</b> Wed,	<b>#:</b> 1601 Jan 18	17805 2023
236 ★ 8251 = 209 ★ 76 =	446 97 341 • • • • 099 • • • • 099 • • • • • • • • • • • • • • • • •	392 * * * * * * * * * * * * *	12 ← 123 62 49 → 83			Pe Pea	ak-Hou k 15-M	r: 7:45 lin: 7:4	AM	- 8:45 / 8:00	AM AM			4.7 ← 4.9 0 5.7 → 13.2	13 1 1 1 1 1 2 1 1 3 4 32	4.3 5 0 4 42 6	€ 0 ↔ ← 0 € 0 →	0
11		• [ • ] • [	8		-	8		Ļ			÷-			0 0 0			► 0 ► 0 ► 0	
← 3 N/A → + 7	+ → + - + - + - + - + - +		► N/A ►		-		≁ → →		1	<b>↑</b>				N/A			⊾ ⊨ N/A	
15-Min Count Period		S Mil (North	ler St bound)			S Mil (South	ler St bound)			E Co (Eastb	ok St ound)			E Co (West	ok St bound)		Total	Hourly Totals
7:00 AM	Left 8	29	Right 2	0	Left 0	27	Right 10	<b>U</b>	Left 6	6	Kight 16	0	Left 1	Ihru 3	Right 0	0	108	
7:15 AM 7:30 AM	12 11	27 55	6 4	0 0	2 1	62 76	6 18	0 0	9 15	9 9	15 13	0 0	1 4	5 12	1 3	0 0	155 221	
7:45 AM	16	65	6	0	0	107	23	0	20	13	20	0	24	21	7	0	322	806
8:15 AM	20	89	7	0	4	73	25	0	23	15	26	0	7	11	0	0	304	1151
8:30 AM 8:45 AM	19 24	69 45	4	0	2	71	30	0	14 9	12 8	17 21	0	5 2	10 18	2	0	247 238	1177 1093
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		Tai	tal
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	Ldi
All Vehicles Heavy Trucks	64 0	260 20	24 0	0	0	428 8	92 0	0	80 8	52 0	80 24	0	96 0	84 0	28 0	0	12	88 0
Buses Pedestrians Bicycles Scooters	0	4 0	0		0	0 4	0		0	12 0	0		0	4 0	0		2	0 
Comments:																		

LOCATION: S CITY/STATE:	5 Mille Santa	r St E Maria,	E Cook S , CA	St											QC DATE:	C <b>JOB</b> # Wed,	<b>‡:</b> 1601 Jan 18	17806 2023
460 ← 173 . 67 419 → 179	590 149 42 149 42 1	667 2 19 4 6 22 6 22 678	8 ← 177 141 28 → 108			Pe Pea	ak-Hou k 15-M Data TH	ir: 4:00 lin: 4:0		- 5:00 I 4:15	PM PM			35 ← 06 15 33 → 67	19 13 12 		≥ 0 + 1 ⇒ 21 ► 0 + 1	17
16		• [ • ]	10		-	18		Ļ		ļ	<u>₽</u>	-		1 0 0			0 1 0	
← 3 N/A → + 7			► N/A ►		-		★		٦	↑ ↑ [ 	<b>1</b>	-		N/A			► N/A	
15-Min Count Period		S Mil (North	ler St bound)			S Mil (South	ler St bound)			E Co (Eastb	ok St ound)			E Co (West	ok St bound)		Total	Hourly Totals
A-00 DM	Left	Ihru	Right	U	Left	112	Kight	U	Left	I hru	Right	U	Left	Ihru	Right	U	405	
4:15 PM	45	135	9	0	4	93	27	0	54	24	45	0	10	48	1	0	495	
4:30 PM	37	117	4	0	4	113	33	0	46	19 13	43	0	3	26	2	0	447	1864
5:00 PM	37	151	5	0	7	98	16	1	63	20	51	0	3	20	0	0	473	1842
5:15 PM	36	109	6	0	4	109	22	0	51	19 25	47	0	9 15	15	5	0	432	1779 1755
5:45 PM	40	96	1 5	0	2	88	18	0	45 32	25 16	20 32	0	15	19	2	0	339	1667
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		-	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	al
All Vehicles Heavy Trucks	196 4	492 8	28 0	0	28 0	452 0	220 0	0	144 0	44 0	164 16	0	32 0	164 4	16 0	0	19 3	80 2
Buses Pedestrians Bicycles Scooters	0	20 0	0		0	12 0	0		0	36 0	0		0	32 0	0		10 0	00
Comments:																		

PROJECT TRIP GENERATION CALCULATION WORKSHEET

#### Associated Transportation Engineers #24017 Trip Generation Worksheet - Proposed Uses

			SEARS	BUILDIN	IG REMO	DEL PR	OJECT -	PROPO	SED USE	S						
		Internal-Trip	Α	DT			AM PEA	K HOUR	1				PM PEA	K HOUR		
Use	Size	Factor	Rate	Trips	Rate	Trips	In %	Trips	Out %	Trips	Rate	Trips	In %	Trips	Out %	Trips
PROPOSED																
Grocery Store (a)	50,989 SF	1.00	93.84	4,785	2.86	146	59%	86	41%	60	8.95	456	50%	228	50%	228
Apparel Store #1 (b)	27,242 SF	1.00	66.40	1,809	1.00	27	80%	22	20%	5	4.12	112	51%	57	49%	55
Apparel Store #2 (b)	23,651 SF	1.00	66.40	1,570	1.00	24	80%	19	20%	5	4.12	97	51%	49	49%	48
Totals	101,882 SF			8,164		197		127		70		665		334		331

(a) Trip generation based on ITE rates for Supermarket (ITE #850).

(b) Trip generation based on ITE rates for Apparel Store (ITE #876).

<b>GROCERY STORE PASS-BY &amp; PRIMARY TRIPS</b>	<b><u>ADT</u></b>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	PM Total	<u>PM In</u>	<u>PM Out</u>
Commercial External Trips - Grocery Store	4,785	146	86	60	456	228	228
24% Pass-By Trips - Applied to Grocery Store	1,148	35	21	14	109	55	54
76% Primary Trips - Remainder Grocery Store	3,637	111	65	46	347	173	174
APPAREL STORE PASS-BY & PRIMARY TRIPS	<u>ADT</u>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	<u>PM Total</u>	<u>PM In</u>	PM Out
Commercial External Trips - Apparel Store	3,379	51	41	10	209	106	103
19% Pass-By Trips - Applied to Apparel Store	642	10	8	2	40	20	20
81% Primary Trips - Remainder Apparel Store	2,737	41	33	8	169	86	83
TOTAL PASS-BY TRIPS	<u>ADT</u>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	PM Total	<u>PM In</u>	<u>PM Out</u>
Grocery Store	1,148	35	21	14	109	55	54
Apparel Store	642	10	8	2	40	20	20
Total Pass-By Trips	<b>1,790</b>	<b>45</b>	<b>29</b>	<b>16</b>	<b>149</b>	<b>75</b>	<b>74</b>
TOTAL EXTERNAL PRIMARY TRIPS	<b>ADT</b>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	<u>PM Total</u>	<u>PM In</u>	<u>PM Out</u>
Grocery Store External	3,637	111	65	46	347	173	174
Apparel Store External	2,737	41	33	8	169	86	83
Total External Trips	<b>6,374</b>	<b>152</b>	<b>98</b>	<b>54</b>	<b>516</b>	<b>259</b>	<b>257</b>

	Ŭ	303														
			SEAR	S BUILDI	NG REM	ODEL PF	ROJECT	- EXISTI	NG USES	S						
		Internal-Trip	A	DT			AM PEA	K HOUR					PM PEAI	K HOUR		
Use	Size	Factor	Rate	Trips	Rate	Trips	In %	Trips	Out %	Trips	Rate	Trips	In %	Trips	Out %	Trip
EXISTING																
Sears (a)	101.882 SF	1.00	37.01	3.771	0.84	86	62%	53	38%	33	3.40	346	48%	166	52%	180
				-,		50	/ -		/-						/ •	

(a) Trip generation based on ITE rates for Shopping Center (ITE #820).

<b>SEARS PASS-BY &amp; PRIMARY TRIPS</b>	<u>ADT</u>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	<u>PM Total</u>	<u>PM In</u>	<u>PM Out</u>
Commercial External Trips - Grocery Store	3,771	86	53	33	346	166	180
19% Pass-By Trips - Applied to Sears	716	16	10	6	66	32	34
81% Primary Trips - Remainder Sears	3,055	70	43	27	280	134	146
TOTAL PASS-BY TRIPS	<u>ADT</u>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	<u>PM Total</u>	<u>PM In</u>	<u>PM Out</u>
Sears	716	16	10	6	66	32	34
Total Pass-By Trips	<b>716</b>	<b>16</b>	<b>10</b>	6	<b>66</b>	<b>32</b>	<b>34</b>
TOTAL EXTERNAL PRIMARY TRIPS	<u>ADT</u>	<u>AM Total</u>	<u>AM In</u>	<u>AM Out</u>	<u>PM Total</u>	<u>PM In</u>	<u>PM Out</u>
Grocery Store External	3,055	70	43	27	280	134	146
Total External Trips	<b>3,055</b>	<b>70</b>	<b>43</b>	<b>27</b>	<b>280</b>	<b>134</b>	<b>146</b>

Associated Transportation Engine Trip Generation Worksheet - Net F	∍ers #24017 External Trips														
SEARS BUILDING REMODEL PROJECT - NET EXTERNAL TRIPS															
,		ADT		AM PEAK HOUR						PM PEAK	HOUR				
Use	Size	Rate T	rips	Rate	Trips	In %	Trips	Out %	Trips	Rate	Trips	In %	Trips	Out %	Trips
PROPOSED															
Grocery Store & Apparel Stores (a)	101,882 SF	6	,374		152		98		54		516		259		257
EXISTING															
Sears (a)	101,882 SF	3	,055		70		43		27		280		134		146
NET TOTALS		3	3,319		82		55		27		236		125		111

(a) Only includes external primary trips.

CITY OF SANTA MARIA APPROVED AND PENDING PROJECTS LIST





# City of Santa Maria **MAJOR DEVELOPMENTS (JANUARY 2024)**

COMMERCIAL INDUSTRIAL

MIXED USE/OTHER

### Residential

**309 Mill Apartments** 309 E Mill St 23 unit apartments

6 200 Mill Apartments 200 W Mill St 20 unit apartments

**Vino Bella Apartments** 120 W Chapel St 32 unit apartments

**D** Bellecrest Residences (Paradiso) 1571 E Main St 142 single family senior homes

Heritage Walk Lofts 201 Town Center West 102 residential units

### Commercial

Preisker Commercial Center N Broadway at Preisker Ln 108 rm hotel, drive thru rest, retail

Quick Quack Carwash **W**899 N Broadway 3,588 sq.ft. drive-through car-wash

**9** Starbucks at Home Motors 1313 E Main St Coffee shop & drive-thru

**Nutrien AG Solutions** 1300 block of West Mai 1300 block of West Main Street Outdoor storage & truck repair

facility B Main Miller Retail Building 226 E Main St Lower lever grocery store &

multiple commercial tenants on upper level

## Industrial

3 Donahue Truck Center Preisker Lane Rental facility, truck sales & service

Bonita Packing Expansion 1850 W Stowell Rd 173,720 sq ft cooler addition

Maxco Box Facility 1550 W Stowell Rd 60,000 sq ft & outdoor storage

**Seaside Warehouse** La Brea Ave 40,854 sq ft facility

### **Cook Street Apartments** N of Cook & E of McClelland 114 unit apartments

Heritage View 124 S College Dr SB35 project including 40 senior units and 79 family units

Oakley Court Apartments 600 Block S Oackley Ct 30 unit apartments

Blosser Ranch NE/c Blosser Rd & W Battles Rd 338 "for rent" single family homes with 329 ADUs & 832 apartments



Centennial Square SW/c Miller St & Plaza Dr 184 unit affordable apartments

**Barcellus Senior Apartments** 502 E Barcellus Ave 80 unit senior apartments

Centennial Gardens SW/c Battles St & Depot St 160 unit affordable apartments

**Bradley Commercial** 1423 S Bradley Rd Drive-thru coffee shop & carwash

Home Motors 1004 E Battles Rd 52,000 sq ft auto dealership

Mister Carwash 1925 S Broadway Drive-thru carwash

**35** Splash N Dash Lot 8 Enos Ranch 8,200 sq ft car wash **Starbucks Drive-Thru Coffee** 1202 W. Betteravia Drive-thru only

Planes of Fame 3335 Corsair Cir Air museum with two aircraft hangars

**30** SM Cooler & Box Facility 1767 and 1795 A St 130,000 sq ft cooler & box facility

**45** SM Airport Foxenwood Self Storage 3335 Corsair Circle 101,450 sq ft mini-warehouse facility

**3** Windset Farms Greenhouse 1650 Black Rd 4.3 mil sqft greenhouse & 93k bldg

**37 Hardy Diagnostics** 1291 W McCoy Lane 36,400 sq ft manufacturing, warehouse & office



**33** 2811 Center 2811 Airpark Dr 51,200 sq ft of office in 2 bldgs

## **Mixed Use/Other**

2 Holiday Inn Express & Suites Roemer Court Four story hotel

**B** Gateway Mixed Use 101 N Broadway 33,700 sq ft 4 story mixed use bldg

Alvin Newton Apartments SEC Main St and Broadway 5 story mixed use bldg



Blosser Rd & W Battles Rd 126 apts & 16k sq ft retail

**33** Betteravia Plaza W Betteravia Rd at SMVRR 443 apts & 291,278 sq ft retail/office





**A** Richards Ranch Annexation Orcutt Expressway & Union Valley Parkway 43.75 acres of annexation

Vandenberg Senior Residence 52 unit senior apartment addition

Avante Apartments SW/c of Carmen Ln & S Blosser Rd 86 unit apartments



 Northman Residential (Skyview)
 SM Way btw Sunrise Dr & E Dauphin St 63 single family residences

Skylight Homes 3170 Santa Maria Way 49 single family homes

Park Edge Apartments SE/c Santa Maria Way & S Miller St 140 apts & 5,435 sq ft comm

> Elements Apartments (Lakeview Mixed Use Project) NW corner of Orcutt Expressway & Skyway Dr 152 apartments & 9,800 sqft commercial



### City of Santa Maria

### MAJOR DEVELOPMENTS (JANUARY 2024)

	Duciest	Dreicker Commercial Conter	Cataman	Commercial	<b>F</b> #a #a	Ammuna	Otatua
	Project		Category	Commercial		Approved	Status
4	Description	108 rm hotel and two drive-thru fast food restaurants totaling 8,300 sq. f	t Acreage	5	PD2015-0011	5/18/2016	review (PD2023-0005).
	Location	NW/c N. Broadway and Preisker Ln	District	PD-f/C-2	TR2016-0001	9/7/2016	
	APN(s)	128-002-048, -049 & -050	Planner	Cody Graybehl	A2019-0004	2/18/2019	
	Contact	Jody Walker Belsick, Applicant, 702-786-1829			PD2023-0005	Pending	
	Project	Holiday Inn Express & Suites	Category	Mixed/Other	File #s	Approved	Status
•	Description	New 4 story hotel, wood construction	Acreage	2.13	PD2022-0001	Pending	Planning permits under review.
2	Location	Roemer Ct.	District	CM	GPZ2022-0001	Pending	
	APN(s)	128-003-047, -048	Planner	Cody Graybehl			
	Contact	Prakash Patel, Applicant, 669-333-1880					
	Project	Donahue Truck Center	Category	Industrial	File #s	Approved	Status
	Description	Rental facility, truck sales and service	Acreage	1.53	PD2022-0016	Pending	Planning permits under review.
3	Location	Preisker Lane	District	PD-F/CM			
	APN(s)	128-003-008	Planner	Cody Graybehl			
	Contact	Thele-Donahue, LLC					
	Project	Quick Quack Drive-Through Carwash	Category	Commercial	File #s	Approved	Status
	Description	3,588 sq.ft. drive-through carwash, point-of-sale canopy & vacuum enclo	) Acreage	1.34	PD2023-0018	Pending	Planning permit under review
4	Location	899 N Broadway	District	PD/C-2	U2023-0018	Pending	
	APN(s)	121-071-015	Planner	Greg Vine			
	Contact	Erika Hernandez, Applicant, 818-398-5179					
	Project	309 Mill Apartments	Category	Residential	File #s	Approved	Status
	Description	Construct a 23 unit, 9750 sq. ft. apartment building	Acreage	0.2	DT2020-0015	Pending	Planning permits under review.
5	Location	309 E Mill St	District	DTSP - Bungalow Di	strict		
	APN(s)	121-193-011	Planner	Greg Vine			
	Contact	Jason Heyward, Consultant, 805-928-8948					
	Project	200 Mill Apartments	Category	Residential	File #s	Approved	Status
	Description	Construct a 20 unit, 3-story building	Acreage	0.17	DT2022-0019	Pending	Planning permits under review.
6	Location	200 W Mill Street	District	DTSP- Bungalow			
	APN(s)	119-273-007	Planner	Frank Albro			
	Contact	Halsell Builders - Jason Heyward, Applicant, 805-928-8948					
	Project	Vino Bella Apartments	Category	Residential	File #s	Approved	Status
	Description	Construct a 32 unit, 3-story apartment building	Acreage	0.3	DT2020-0017	12/16/2020	Building permits submitted.
7	Location	120 W Chapel St	District	DTSP - Bungalow Di	strict		
-	APN(s)	119-276-015	Planner	Frank Albro			
	Contact	Ben Nikfarjam, Applicant, 310-215-4882					
	Project	Gateway Mixed Use	Category	Mixed/Other	File #s	Approved	Status
	Description	33,700 sq. ft., four-story mixed use development	Acreage	0.3	DT2017-0033	1/16/2018	Under construction.
8	Location	101 N. Broadway	District	DTSP - Gateway	A2019-0032	9/4/2019	
	APN(s)	119-276-019	Planner	Frank Albro			
	Contact	Ben Nikfarjam, Developer, 310-251-4882					

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	Project	Starbucks at Home Motors	Category	Commercial	File #s	Approved	Status
	Description	Coffee shop and drive-thru	Acreage	5.81	PD2021-0011	11/16/2022	Under construction.
9	Location	1313 E Main St	District	PD/C-2	U2021-0019	11/16/2022	
	APN(s)	128-120-003	Planner	Carol Ziesenhenne			
	Contact	Jacob Weintraub, Applicant, 805-441-0332					
	Project	Bellecrest Residences (Paradiso)	Category	Residential	File #s	Approved	Status
	Description	142 single-family senior residential homes	Acreage	14.58	GPZ 2022-0003	Pending	Planning permits under rev
10	Location	1571 E Main Street	District	PD/R-1	PD2022-0008	Pending	
	APN(s)	128-052-014 & 023	Planner	Frank Albro	PD2022-0009	Pending	
	Contact	Cam Boyd, Applicant, 805-556-3060x164			TR2022-0007	Pending	
	Project	Nutrien AG Solutions	Category	Commercial	File #s	Approved	Status
4.4	Description	Phased Expansion (Phase 1 outdoor storage and property improvements, Phase 2 a new 6,7000 sqft truck repair facility)	Acreage	4.42	PD2023-0014	Pending	Planning permits under rev
11	Location	1300 block of West Main Street	District	PD/CM	A2022-0013	Pending	
	APN(s)	117-180-030	Planner	Greg Vine		-	
	Contact	Nutrien AG Solutions, Applicant, 805-922-5848					
	Project	Alvin Newton Apartments	Category	Mixed Use/ Other	File #s	Approved	Status
	Description	5 story mixed-use (1 floor commercial and 4 floors apartments)	Acreage	1.49	DT2022-0022	10/3/2023	Planning permit approved.
12	Location	SWC Main St. and Broadway	District	DTSP- Gateway			
	APN(s)	125-320-050	Planner	Carol Ziesenhenne			
	Contact	The Vernon Grop, Applicant, 805-963-1244					
	Project	Main Miller Retail Building	Category	Commercial	File #s	Approved	Status
	Description	Lower level grocery store & multiple commercial tenants on upper level	Acreage	5.4	DT2023-0012	Pending	Planning permit under revie
13	Location	226 E Main St	District	DT SP Town Center			
	APN(s)	125-320-038	Planner	Carol Ziesenhenne			
	Contact	Shasta2020, LP - Mark Gabay, Applicant, 310-247-0900					
	Project	Heritage Walk Lofts	Category	Residential	File #s	Approved	Status
	Description	Re-purpose structure for 102 residential units	Acreage	1.29	DT2022-0018	3/7/2023	Planning permit expiration
14	Location	201 Town Center West	District	DTSP- Town Center			3/7/2026.
• •	APN(s)	123-280-003	Planner	Carol Ziesenhenne			
	Contact	Vernon Property Group, LLC, Applicant					
	Project	Cook Street Apartments	Category	Residential	File #s	Approved	Status
	Description	Six story building to accommodate up to 114 residential apartment units	Acreage	0.44	DT2022-0017	11/15/2023	Planning permit approved.
15	Location	N of Cook Street and E of McClelland Street	District	DTSP - Gateway			
	APN(s)	125-320-018, -019	Planner	Frank Albro			
	Contact	Brian Schwartz, Principal Planner, 805-934-5760					
	Project	Heritage View	Category	Mixed Use	File #s	Approved	Status
	Description	SB 35 project including 40 senior units and 79 family units	Acreage	6.28	SP2023-0008	Pending	Planning permit under revie
16	Location	124 S College Dr	District	PD/CPO & PD/R-2			
	APN(s)	125-044-007	Planner	Cody Graybehl			
	Contact	People's Self Help Housing Corporation, Applicant, 818-849-8613					
	Project	Boone Street Market	Category	Mixed/Other	File #s	Approved	Status
	Description	2,280 sq. ft. addition to market, and 2 new units	Acreage	0.2	GPZ2016-0004	5/2/2017	Building permits issued.
47	Location	501 E. Boone St	District	DTSP - Railroad Loft	SPZ2016-0003	5/2/2017	
17	APN(s)	125-114-015	Planner	Carol Ziesenhenne	DT2016-0040	8/21/2017	
	Contact	Brian Schwartz, Consultant, 805-934-5760			A2019-0006	3/4/2019	
					A2020-0012	6/8/2020	
	Project	Oakley Court Apartments	Category	Residential	File #s	Approved	Status
	Description	30 apartment units with on-site manager's unit	Acreage	2.1	GPZ2019-0001	10/1/2019	Planning permit expiration
18	Location	600 Block S. Oakley Ct	District	PD/R-3	PD2019-0002	7/17/2019	7/17/2024.
	APN(s)	123-140-036	Planner	Frank Albro	A2022-0010	1/18/2023	
	Contact	Lupe & Gustavo, Applicant, 805-937-1108					

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	Project	Bonita Packing Expansion	Category	Industrial	File #s	Approved	Status
	Description	173,270 sq. ft. cooler addition in 4 phases	Acreage	45.4	PD2012-0007	5/1/2013	Phase 1 (45,935 sq. ft.) is co
19	Location	1850 W. Stowell Rd	District	PD/CM			
	APN(s)	117-820-028	Planner	<u>Dana Eady</u>			
	Contact	John Smith, Engineer, 805-466-5660					
	Project	Maxco Box Facility	Category	Industrial	File #s	Approved	Status
	Description	Construct a new 60,000 sq. ft. box facility and outdoor storage yard	Acreage	19.8	PD2021-0007	9/21/2022	Building permits submitted.
20	Location	1550 W Stowell Rd	District	PD/CM-AG	U2021-0020	9/21/2022	permit expiration on 9/21/20
	APN(s)	117-820-015	Planner	Cody Graybehl			
	Contact	Steve Rigor, Applicant, (503) 477-8328 x 112					
	Project	Seaside Packaging Warehouse	Category	Industrial	File #s	Approved	Status
	Description	40, 854 square-foot packaging warehouse	Acreage	6.18	U2021-0002	3/16/2022	Under construction.
21	Location	La Brea Avenue	District	M-2			
	APN(s)	117-240-034	Planner	Cody Graybehl			
	Contact	Suzanne D. Winslow, Applicant, (805) 544-9700					
	Project	Blosser Ranch	Category	Mixed/Other	File #s	Approved	Status
	<b>-</b>	Construct 338 "for rent" single-family residences with 329 ADUs as well			PD2023-0002	11/1/2023	Building permits submitted.
	Description	as 832 apartments totalling 1,499 units	Acreage	155.5			permit expiration on 10/18/2
	Location	NE/c of S. Blosser Rd and W. Battles Rd	District	Blosser SE SP	TR2023-0001	11/1/2023	(PD2023-0006, PD2023-00 Planning permit expiration of
	APN(s)	117-240-028	Planner	Carol Ziesenhenne	PD2023-0006	10/18/2023	11/01/2026 (PD2023-0002,
	Contact	Laurie Tamura. Consultant. 805-934-5760			TR2023-0002	11/1/2023	0007).
00					PD2023-0007	11/1/2023	
22					PD2023-0011	10/18/2023	
					PD2022-0013	6/7/2023	
					PD2022-0007	5/3/2023	
					PD2022-0006	5/3/2023	
					TR2019-0003	10/20/2020	
					SP72016-0002	10/20/2020	
					GPZ2016-0003	10/20/2020	
	Proiect	Vandenberg Senior Residences	Category	Residential	File #s	Approved	Status
	Description	52 unit senior apartment addition	Acreage	4.9	PD2017-0002	7/18/2018	Building permit approved. P
23	Location	1314 S. Broadway	District	PD/C-1	A2021-0008	11/17/2021	permit expiration on 7/18/20
20	APN(s)	128-065-008	Planner	Cody Gravbehl	A2023-0004	Pendina	
	Contact	Barry Williams Architect 805-459-7353					
	Project	Centennial Square Anartments	Category	Residential	File #s	Approved	Status
	Description	184 affordable apartments	Acreage	6.35	PD2020-0009	8/4/2021	Under construction.
24	Location	SW/c Miller St and Plaza Dr	District	PD/R-3		0, 1,2021	
24	APN(s)	128-066-003	Planner	Carol Ziesenbenne			
	Contact	Brian Schwartz, Consultant, 805-034-5760					
	Project	Barcellus Senior Anartments	Category	Residential	File #s	Annroved	Status
	Description	80 unit senior apartments	Acreage	2.3	GPZ2016-0002	12/7/2016	Planning permit expiration o
25	Location	502 F. Barcellus Ave	District	2.0 PD/R-3	PD2022-0015	2/1/2023	2/1/2026.
ZJ	APN(s)	128-067-032 -033 -034	Planner	Cody Graybehl			
	Contact	Brian Schwartz, Consultant, 805-034-5760	i lanner				
	Project	Westgate Village	Category	Mixed/Other	File #s	Approved	Status
	Description	126 multifamily units and 16 000 sq. ft. retail (including gas station)	Acreage	7.6	PD2007-012	7/2/2008	Planning permit under revie
	Location	NW/c S Blosser Rd and W Battles Rd	District	PD/CC	A2017-0029	2/7/2018	
	APN(s)	117-240-046 -045	Planner	Carol Ziesenbenne	A2018-0023	1/16/2019	
	Contact	Craig Minus, Developer, 805,062,2121		Jaioi Lieseillieillie	Δ2020-0023	5/20/2019	
26	Contact	Orarg Willius, Developer, 000-302-2121			Δ2021-0003	3/11/2020	
					CD72022 0002	Donding	
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	Project	Bradlov Commorcial	Category	Commorcial	Eile #s	Approved	Status
	Description		Acreage	2.7	PD2023_0019	Pendina	Planning permit under revi
07			Acreage	5.7	112023-0019	Ponding	
21	Location	1423 S Bradley Rd	District	PD/C-2	02023-0019	renuing	
	APN(s)	128-139-016	Planner	Cody Graybehl			
	Contact	Santa Maria South Bradley Investments, LLC, Applicant, 530-668-1000					
	Project	Centennial Gardens	Category	Residential	File #s	Approved	Status
	Description	Construct 160 affordable apartment units	Acreage	8.36	PD2020-0006	11/18/2020	Under construction.
28	Location	SW/c Battles and Depot	District	PD/R-3			
	APN(s)	118-010-058	Planner	Frank Albro			
	Contact	Brian Schwartz, Consultant, 805-934-5760					
	Project	Home Motors	Category	Commercial	File #s	Approved	Status
	Description	52,000 sq. ft. auto dealership	Acreage	7.2	PD2018-0004	5/16/2018	Building permits submitted
29	Location	1004 E. Battles Rd	District	Enos Ranchos SP			
	APN(s)	128-189-002	Planner	Carol Ziesenhenne			
	Contact	Jacob Weintraub, Consultant, 805-441-0332					
	Project	Santa Maria Cooler and Box Facility	Category	Industrial	File #s	Approved	Status
	Description	Multi-phased 130,000 sq. ft. AG cooler and box facility	Acreage	11.48	PD2023-0013	Pending	Planning permit under revi
30	Location	1767 and 1795 A St	District	PD/M-1 - Area 9 SP			
	APN(s)	117-820-022 & 117-820-036	Planner	Cody Graybehl			
	Contact	Gil Palacios, Architect, 805-928-8008					
	Project	Windset Farms Greenhouses 7-9	Category	Industrial	File #s	Approved	Status
_	Description	4.3 mil sq. ft. greenhouse and 93,000 sq. ft. bldg.	Acreage	49	PD2017-0009	Pending	Planning permit under revi
31	Location	1650 Black Rd	District	Area 9 SP			
	APN(s)	117-310-018	Planner	<u>Dana Eady</u>			
	Contact	Brian Schwartz, Consultant, 805-934-5760					
	Project	Avante Apartments	Category	Residential	File #s	Approved	Status
	Description	86 unit apartment complex	Acreage	3.91	PD2021-0013	11/16/2022	Building permits submitted
32	Location	SW/c of Carmen Lane and South Blosser Road	District	PD/R-3	TR2022-0002	11/16/2022	permit expiration 11/16/20
	APN(s)	117-770-047	Planner	Cody Graybehl			
	Contact	Steve Simoulis, Applicant, 805-440-9876					
	Project	Betteravia Plaza	Category	Mixed/Other	File #s	Approved	Status
	Description	Up to 443 units and 291,278 sq. ft. of retail/office	Acreage	55.2	DA2015-0001	2/2/2016	Planning permit expiration
22	Location	NW/c of W. Betteravia Rd & SMVRR tracks	District	Multiple	TR2016-0007	11/21/2018	02/07/2026.
33	APN(s)	117-990-001	Planner	Carol Ziesenhenne	GPZ2021-0002	2/21/2023	
	Contact	Dan Blough, Consultant, 805-680-9666			PD2021-0006	2/7/2023	
					PD2022-0011	2/7/2023	
	Project	Mister Carwash	Category	Commercial	File #s	Approved	Status
	Description	Drive-thru carwash with on-site office and storage	Acreage	0.92	PD2023-0008	9/6/2023	Planning permit expiration
34	Location	1925 S. Broadway	District	PD/C-2 - Entrada SP			09/06/2026.
	APN(s)	117-500-029 and 117-500-012	Planner	Greg Vine			
	Contact	Lauren Smith, Applicant, 713-449-9447					
	Project	Splash N Dash	Category	Commercial	File #s	Approved	Status
	Description	8,200 sq ft carwash	Acreage	1.6	PD2018-0005	9/4/2019	Grading permits submitted
35	Location	Lot 8	District	Enos Ranchos SP			permit expiration on 9/4/20
	APN(s)	128-189-008	Planner	Carol Ziesenhenne			
	Contact	Jacob Weintraub, Consultant, 805-441-0332					
	Project	Starbucks Drive-Thru Coffee	Category	Commercial	File #s	Approved	Status
	Description	1,300 sq. ft. drive-thru only	Acreage	0.51	PD2023-0009	Pending	Planning permit under revi
36	Location	1202 W. Betteravia	District	PD/C-2	U2023-0007	Pending	
00	APN(c)	111-400-032	Planner	Cody Graybehl		5	
	Contact	Inn Collete Architect 805 649 1224 ovt 20		ody oraybern			
	Contact	Jane Collete, Alchitect, 000-046-1234 ext. 20					

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	Project	Hardy Diagnostics	Category	Industrial	File #s	Approved	Status
	Description	36,400 sq. ft. manufacutring, warehouse and office building	Acreage	1.96	PD2023-0003	6/21/2023	Planning permit expiration
37	Location	1291 W. McCoy Lane	District	PD/M-1			
•••	APN(s)	111-051-011	Planner	Frank Albro			
	Contact	Pamela Ricci, Applicant, 805-543-1794					
	Project	2811 Center	Category	Industrial	File #s	Approved	Status
	Description	51.200 sq. ft. of office in 2 buildings	Acreage	7	PD2017-0003	6/7/2017	One 25,600 sq. ft. building
38	Location	2815 Airpark Dr	District	PD/M-1	TR2017-0002	3/21/2018	constructed. Second building
	APN(s)	111-231-003	Planner	Carol Ziesenhenne	A2022-0001	1/25/2022	
	Contact	Steve Simoulis, Developer, 805-541-9004					
	Project	Santa Maria Studios	Category	Residential	Files #s	Approved	Status
	Description	358 senior, affordable & market rate units (Phase 1= 160 + Phase 2 =	= 1{Acreage	5.5	SP2021-0003	2/21/2021	Phase 1 is under construct
39	Location	2660 Santa Maria Way. Santa Maria. CA	District	PD/C-2	PD2023-0004	Pending	Planning permit under revie
	APN(s)	128-090-011	Planner	Cody Gravbehl			Phase 2.
	Contact	AMG & Associates 11 C 818-380-2600		<u> </u>			
	Project	Park Edge Apartments	Category	Mixed Use/Other	File #s	Approved	Status
	Description	140 apt units, clubhouse and 5,435 sq. ft, multi-tennant commercial	Acreage	7.45	PD2020-0008	6/16/2022	Building permits submitted.
40	Location	2770 Santa Maria Way	District	PD/C-2 & PD/R-3	U2020-0012	6/16/2022	permits expiration on 6/16/2
τu	APN(s)	128-000-022 _023 & 100-010-030	Planner	Cody Graybehl		•, ••, =•==	
	Contact	Brian Schwartz, Consultant, 805-034-5760	i lannei	Couy Graybern			
	Project	Northman Posidential (Skyview Homes)	Category	Posidontial	File #s	Annroved	Status
	Description	63 single family residences	Acreade	13.2	GP72018-0004	8/6/2019	Under construction
11	Location	Santa Maria W/v htw Suprice $Dr = 8 E Dauphin St$	District	10.2 DD/D 1	TR2018-0003	7/16/2010	
41			District			6/10/2013	
	Contact	Brian Schwertz, Consultant	Fiannei		A2024 0042	4/28/2013	
	Project	Skylight Homes	Category	Residential	File #s	Annroved	Status
	Description	40 single family homes in 50 percels	Acreage		GP72021_0001	1/1/2022	Planning permits expiration
12	Location	2170 Sente Marie Way	District	0.09 D/D 1		1/17/2022	Building permits submitted.
42		100 010 012	District	FD/R-I	TP2022-0006	1/17/2023	
	APIN(S)	109-010-012 Obered Element 205 540 0405	Planner	Cody Graybeni	182022-0005	1/1//2023	
	Draiaat	Sheryi Flores, Applicant, 805-540-2465	Cotogony	Commercial	File #e	Approved	Statua
	Project	Air Museum with two sizes of honores (EC C2E on the 2 12 001 on the)	Calegory	Commercial		Donding	Dianning permit under revie
40	Description	Air Museum with two aircrait hangars (56,635 sq.it. & 12,801 sq.it.)	Acreage	23.94	PD2023-0017	Penaing	r ianning permit under revie
43	Location	3335 Corsair Cir	District	PD/AS-II & PD/AS-II			
	APN(s)	111-231-011	Planner	Carol Ziesenhenne			
	Contact	Jane Hinton, Applicant					
	Project	Elements Apartments (Lakeview Mixed Use Project)	Category	Mixed Use	File #s	Approved	Status
	Description	152 apartment units and approx. 9,800 sqft of commercial space	Acreage	4	PD2018-0008	4/2/2019	Under construction
44	Location	NW corner of Orcutt Expressway & Skyway Drive	District	PD/R-3			
	APN(s)	111-100-008 & 111-100-009	Planner	Frank Albro			
	Contact	Urban Planning Concepts, Applicant, 805-934-5760					
	Project	Santa Maria Airport Foxenwood Self Storage	Category	Industrial	File #s	Approved	Status
	Description	Mini-warehouse facilty	Acreage	608.01	PD2022-0017	1/17/2024	Planning permit expiration
45	Location	3335 Corsair Circle	District	AA,PD/AS-1	SPZ2022-0001	1/17/2024	
	APN(s)	111-231-011	Planner	Frank Albro			
	Contact	Santa Maria Public Airport District, 805-922-1726					
	Project	Richards Ranch Annexation	Category	Mixed Use	File #s	Approved	Status
	Description	Annexation of 43.75 acres	Acreage	43.75	AN2021-0001	Pending	Annexation application und
46	Location	Orcutt Expressway & Union Valley Parkway	District	Proposed PD/C-2 an	d	-	
	APN(s)	107-250-020, 107-250-019, 107-250-021, & 107-250-022		PD/R-3			
	Contact	Urban Planning Concepts, Applicant, 805-934-5760	Planner	Dana Eady			
				<u> </u>			

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on 1/17/2026. :d.	
view	
n 1/17/2027.	
nder review	

APPROVED AND PENDING PROJECT TRIP GENERATION SPREADSHEET

#### Associated Transportation Engineers Pending and Approved Projects - Trip Generation Worksheet

	SEARS BUILD	ING REM	ODEL PR	OJECT - CI	UMULA		ITY LIS	6T (#240	017)							
	Land Lloo	<b>C</b> :-		Pass-By	AM Peak								PM	Peak		
Land-03e		Size		Factor	Rate	Trips	In %	Trips	Out %	Trips	Rate	Trips	In %	Trips	Out %	Trips
5	309 Mill Aparments (a)	23	DU	1.00	0.40	9	24%	2	76%	7	0.51	12	63%	8	37%	4
6	200 Mill Aparments (a)	20	DU	1.00	0.40	8	24%	2	76%	6	0.51	10	63%	6	37%	4
7	Vino Bella Apartments (a)	32	DU	1.00	0.40	13	24%	3	76%	10	0.51	16	63%	10	37%	6
10	Bellecrest Residences (j)	142	DU	1.00	-	34	-	11	-	23	-	43	-	26	-	17
14	Heritage Walk Lofts (a)	102	DU	1.00	0.40	41	24%	10	76%	31	0.51	52	63%	33	37%	19
17	Boone Street Market (c)	2,280	SF	1.00	0.59	1	79%	1	21%	0	16.62	38	50%	19	50%	19
15	Cook Street Apartments (h)	-	-	1.00	-	62	-	21	-	41	-	72	-	45	-	27
16	Heritage View (e)	40	DU	1.00	0.50	20	29%	6	71%	14	0.46	18	59%	11	41%	7
16	Heritage View (a)	79	DU	1.00	0.40	32	24%	8	76%	24	0.51	40	63%	25	37%	15
17	Boone Street Market (a)	2	DU	1.00	0.40	1	24%	0	76%	1	0.51	1	63%	1	37%	0
18	Oakley Court Apartments (a)	31	DU	1.00	0.40	12	24%	3	76%	9	0.51	16	63%	10	37%	6
22	Blosser Ranch (d)	-	-	1.00	-	1,448	-	639	-	809	-	1,933	-	1,055	-	878
23	Vandenberg Senior Residence (a)	52	DU	1.00	0.40	21	24%	5	76%	16	0.51	27	63%	17	37%	10
24	Centennial Sqaure Apartments (e)	184	DU	1.00	0.50	92	29%	27	71%	65	0.46	85	59%	50	41%	35
25	Barcellus Senior Apartments (a)	80	DU	1.00	0.40	32	24%	8	76%	24	0.51	41	63%	26	37%	15
8	Gateway Mixed Use (b)	27	DU	1.00	0.37	10	23%	2	77%	8	0.39	11	61%	7	39%	4
8	Gateway Mixed Use (f)	3,300	SF	1.00	2.36	8	60%	5	40%	3	6.59	22	50%	11	50%	11
9	Starbucks at Home Motors (g)	1,800	SF	0.50	85.88	77	51%	39	49%	38	38.99	35	50%	18	50%	17
12	Alvin Newton Apartments (i)	-	-	1.00	-	55	-	22	-	33	-	56	-	35	-	21
	Boot Barn Apartments (b)	101	DU	1.00	0.37	37	23%	9	77%	28	0.39	39	61%	24	39%	15
	Boot Barn Apartments (k)	1,450	SF	1.00	9.57	14	55%	8	45%	6	9.05	13	61%	8	39%	5

(a) Trip generation based on rates for Multifamily Housing Low-Rise (#220).

(b) Trip generation based on rates for Multifamily Housing Mid-Rise (#221).

(c) Trip generation based on rates for Liqour Store (#899).

(d) Traffic Study, ATE, December 2022.

(e) Trip generation based on rates for Affordable Housing (#899).

(f) Trip generation based on rates for Strip Retail Plaza (<40k) (#822).

(g) Trip generation based on rates for Coffee/Donut Shop with Drive-Through Window (#937).

(h) Traffic Study, ATE, February 2023.

(i) Traffic Study, ATE, March 2023.

(j) Traffic Study, ATE, December 2023.

(j) Traffic Study, ATE, December 2023.

(k) Trip generation based on rates for High Turnover Sit-Down Restaurant (#932).

MAIN STREET QUEUING ANALYSIS

#### Intersection: 1: Broadway & Main St

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	Т	Т	R	L	L	Т	Т	R	L	Т
Maximum Queue (ft)	172	176	132	149	171	176	171	221	240	172	334	487
Average Queue (ft)	87	150	97	107	71	143	133	168	163	94	191	333
95th Queue (ft)	193	177	134	166	158	178	184	246	262	160	353	477
Link Distance (ft)			842	842				616	616	616		525
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	210	210			155	450	450				310	
Storage Blk Time (%)				2	0							8
Queuing Penalty (veh)				2	0							15

#### Intersection: 1: Broadway & Main St

Movement	NB	NB	SB	SB	SB	SB
Directions Served	Т	R	L	Т	Т	R
Maximum Queue (ft)	406	75	262	213	189	146
Average Queue (ft)	293	49	175	183	108	51
95th Queue (ft)	412	76	282	225	213	134
Link Distance (ft)	525	525		478	478	478
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			520			
Storage Blk Time (%)						
Queuing Penalty (veh)						

#### Intersection: 2: Town Center Dr & Main St

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	
Directions Served	Т	Т	Т	R	L	Т	Т	Т	L	R	
Maximum Queue (ft)	110	91	51	51	28	149	138	111	67	21	
Average Queue (ft)	64	68	23	21	17	108	87	56	38	11	
95th Queue (ft)	116	100	51	53	40	158	148	121	66	26	
Link Distance (ft)	616	616	616			521	521	521	332	332	
Upstream Blk Time (%)											
Queuing Penalty (veh)											
Storage Bay Dist (ft)				240	240						
Storage Blk Time (%)											
Queuing Penalty (veh)											

#### Intersection: 3: Miller St & Main St

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	Т	Т	R	L	Т	TR	L	Т	TR	L	Т
Maximum Queue (ft)	74	187	200	53	116	202	202	111	110	63	66	112
Average Queue (ft)	37	148	149	22	95	165	130	83	57	48	30	78
95th Queue (ft)	78	204	206	55	126	222	204	131	116	74	69	112
Link Distance (ft)		521	521	521		1066	1066		618	618		563
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	250				230			230			200	
Storage Blk Time (%)												
Queuing Penalty (veh)												

#### Intersection: 3: Miller St & Main St

Movement	SB
Directions Served	TR
Maximum Queue (ft)	30
Average Queue (ft)	29
95th Queue (ft)	30
Link Distance (ft)	563
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Network Summary

Network wide Queuing Penalty: 18

#### Intersection: 1: Broadway & Main St

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	L	Т	Т	R	L	L	Т	Т	R	L	T
Maximum Queue (ft)	143	176	240	332	180	153	155	196	236	208	182	290
Average Queue (ft)	68	139	183	228	146	120	124	136	161	107	111	241
95th Queue (ft)	159	175	251	337	234	180	174	224	265	219	182	287
Link Distance (ft)			842	842				616	616	616		525
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	210	210			155	450	450				310	
Storage Blk Time (%)			2	32	0							0
Queuing Penalty (veh)			4	39	1							0

#### Intersection: 1: Broadway & Main St

Movement	NB	NB	SB	SB	SB	SB
Directions Served	Т	R	L	Т	Т	R
Maximum Queue (ft)	286	53	277	209	164	85
Average Queue (ft)	204	33	179	182	143	46
95th Queue (ft)	282	65	276	221	178	87
Link Distance (ft)	525	525		478	478	478
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			520			
Storage Blk Time (%)						
Queuing Penalty (veh)						

#### Intersection: 2: Town Center Dr & Main St

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	
Directions Served	L	Т	Т	Т	R	L	Т	Т	Т	L	R	
Maximum Queue (ft)	23	157	213	184	118	138	227	128	68	115	22	
Average Queue (ft)	18	138	160	96	66	98	116	81	49	82	20	
95th Queue (ft)	33	179	232	198	115	140	238	136	71	120	22	
Link Distance (ft)		616	616	616			521	521	521	332	332	
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	140				240	240						
Storage Blk Time (%)		6					0					
Queuing Penalty (veh)		2					0					

#### Intersection: 3: Miller St & Main St

Movement	EB	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Directions Served	L	Т	Т	R	L	Т	TR	L	Т	TR	L	Т
Maximum Queue (ft)	159	268	272	97	206	275	242	193	198	169	108	136
Average Queue (ft)	77	175	200	49	135	191	179	128	136	139	74	112
95th Queue (ft)	146	286	311	120	208	297	247	208	208	177	125	149
Link Distance (ft)		521	521	521		1066	1066		618	618		563
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	250				230			230			200	
Storage Blk Time (%)		5				5						
Queuing Penalty (veh)		6				9						

#### Intersection: 3: Miller St & Main St

Movement	SB
Directions Served	TR
Maximum Queue (ft)	113
Average Queue (ft)	85
95th Queue (ft)	120
Link Distance (ft)	563
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

#### Network Summary

Network wide Queuing Penalty: 60

ACCIDENT RATE WORKSHEETS



#### ASSOCIATED TRANSPORTATION ENGINEERS

100 N. Hope Avenue, Suite 4, Santa Barbara, CA 93110 • (805) 687-4418 • (805) 682-8509-F

#### ACCIDENT RATE CALCULATION SHEET - FOR INTERSECTIONS

Project: Project #: Analyst: Date:	Sears Building Remodel Projec 24017 GOM 4/8/2024	t	File Name: Accident Rate Worksheet 3 Year
N/S Street: E/W Street: Weekday:	Elizabeth Street Main Street		
PM Peak Ho	our Entering Volume:	2203	
Peak Hour I	Factor:	10.95	
OI Total Appro Weekend: PM Peak Ho	R aach ADT: our Entering Volume OR ADT:	N/A 75%	(as a percentage of Weekday PM Peak Hour Entering Volume OR ADT)
Period Analyze	d (years):	3	
Number of Acc	idents:	4	
Million Entering	g Vehicle Miles:	24.53 millio	on entering vehicle miles (mevm)
Accident Rate:	Γ	.16 acciden	ts per million entering vehicle miles (mevm)
Intersection Rat California State	te Group: • Average Collision Rate:	<u>107</u> 0.36	ז



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#### ACCIDENT RATE CALCULATION SHEET - FOR INTERSECTIONS

Project: Project #: Analyst: Date:	Sears Building Remodel Project 24017 GOM 4/8/2024		File Name: Accident Rate Worksheet 3 Year
N/S Street:	Miller Street		
E/W Street: Weekday:	Church Street		
PM Peak Hou	ır Entering Volume:	1500	
Peak Hour Fa	actor:	10.95	
OR- Total Approa Weekend:	ch ADT:	N/A	
PM Peak Hou	r Entering Volume OR ADT:	75%	(as a percentage of Weekday PM Peak Hour Entering Volume OR ADT)
Period Analyzed	(years):	3	
Number of Accie	lents:	4	
Million Entering	Vehicle Miles:	16.7 millio	on entering vehicle miles (mevm)
Accident Rate:	0	.24 accider	nts per million entering vehicle miles (mevm)
Intersection Rate	Group: Average Collision Rate:	<u>107</u> 0.36	-

#### INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

Reference 1	-	Broadway/Main Street
Reference 2	-	Town Center Drive/Main Street
Reference 3	-	Miller Street/Main Street
Reference 4	-	Elizabeth Street/Main Street
Reference 5	-	Miller Street/Church Street
Reference 6	-	Broadway/Cook Street
Reference 7	-	Miller Street/Cook Street

#24017 - SI INTERSEC COUNT DA TIME PERIO N/S STREE E/W STREE CONTROL	EARS BUIL TION CAPA TE: DD: T: ET: TYPE:	DING REMODEL P CITY UTILIZATION 01/18/2023 AM PEAK HOUR BROADWAY MAIN STREET SIGNAL	ROJECT WORKSHEE	T				MADY					REF:	01_AM
	NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND													
VOLUMES		L	T R	L	Т	R	L	Т	R	L	Т	R		
(A) EXIS (B) PRC (C) CUN	STING: DJECT-ADDE //ULATIVE:	101 D: 0 107	456 123 0 0 530 129	136 24 161	594 -3 631	106 0 108	81 0 92	455 10 478	78 * 0 * 85 *	165 22 197	434 6 453	133 12 157		
GEOMETRICS														
LANE GEO	METRICS	NORT	TH BOUND L TT R	SOU	TH BOU L T T	JND R	EAS	ST BOUI	ND F R	WES	T BOUND			
					TRAFF		NARIO	DS						
SCENARIO SCENARIO SCENARIO SCENARIO	1 = EXISTI 2 = EXISTI 3 = SHORT 4 = SHORT	NG VOLUMES (A) NG + PROJECT VC F-TERM CUMULAT F-TERM CUMULAT	DLUMES(A+B) IVE (C) IVE + PROJE(	CT VOL	UMES	(B+C)								
			L	EVEL	OF SEF	RVICE	CALCI	JLATIO	NS					
MOVE-	# OF		Ι.	SCE	ENARIO	VOLUM	ES		Ι.	5	SCENARIO	V/C RATIO	<u>S</u>	
MENTS	LANES		1	2	3	4			1	2	3	4	1	
NBL		1600	101	101 456	107 530	107 530			0.063 *	0.063 *	0.067	0.067		
NBR (a)	1	1600	98	98	103	103			0.061	0.061	0.064	0.064		
SBI	1	1600	136	160	161	185			0.085	0 100	0 101 *	0.116 *		
SBL	2	3200	594	591	631	628			0.085	0.185 *	0.101	0.110		
SBR (b)	1	1600	90	90	92	92			0.056	0.056	0.058	0.058		
		2000	04	04	00	00			0.005	0.005	0.000	0.000		
EBL		3200	81	81 465	92 478	92 488			0.025	0.025	0.029	0.029		
EBR (c)	1	1600	62	-03 62	68	68			0.039	0.039	0.043	0.043		
WBL	2	3200	165	187	197	219			0.052 *	0.058 *	0.062 *	0.068 *		
WBT WBR (d)		3200	434	440 116	453 126	459 135			0.136	0.138	0.142	0.143		
					.20	LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
			TOTAL INTER	RSECTIC SCENAR	DN CAPA	ACITY U	TILIZA ERVICE	TION:	0.543 A	0.551 A	0.578 A	0.603 A		
NOTES:	_													
	RTOR:	(a) 20% (b) 15% (c) 20% (d) 20%												
Prin	ted: 03/27/24	4												
EXISTING:			 PARES TO CO		 N (A)					·	·			

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#2401 INTER COUN TIME I N/S S <sup>-</sup> E/W S CONT	7 - SEAF RSECTIO IT DATE: PERIOD: TREET: TREET: ROL TYI	RS BUILD IN CAPAC	NING REMODEL CITY UTILIZATIO 01/18/2023 PM PEAK HOU BROADWAY MAIN STREET SIGNAL	PROJECT N WORKSHE IR	ET									REF:	01_PM
	TRAFFIC VOLUME SUMMARY           NORTH BOUND         SOUTH BOUND         EAST BOUND         WEST BOUND														
NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND															
VOLU		10	L							N	L		<u>N</u>		
(A) (B)	PROJE	NG: CT-ADDEE	200 ): 0	0 0	204 64	-8	0	0	520 26	0	240 94	25	57		
(C)	CUMUL	ATIVE:	203	871 158	234	678	148	177	552	122	269	583	217		
						GE	OMET	RICS							
			NOF	RTH BOUND	SOU	TH BOU	JND	EAS	T BOU	ND	WES	ST BOUND			
LANE	GEOME	TRICS		L TT R		LTT	R		LL TI	ΓR		LL TT R			
						TRAFF		NARIO	)S						
SCEN	ARIO 1 =	= EXISTIN	IG VOLUMES (A	)											
SCEN	ARIO 2 =	= EXISTIN	IG + PROJECT \	/OLUMES(A+	B)										
SCEN	ARIO 3 =	= SHORT	TERM CUMULA	TIVE (C)											
SCEN	ARIO 4 =	SHORT	-TERM CUMULA	IIVE + PROJ	ECT VOL	UMES	(B+C)								
	LEVEL OF SERVICE CALCULATIONS														
MOVE-		# OF	CARACITY		1 2	ENARIO 2	VOLUM	<u>ES</u>		1	2	SCENARIO	V/C RATIO	<u>S</u>	
	5				0 000					0.405	2	0.407	4		
NBL			1600	20	0 200 8 808	203	203			0.125	0.125	0.127	0.127		
NBR	(a)	1	1600	10	4 104	111	111			0.255	0.065	0.069	0.272		
SBL		1	1600	20	4 268	234	298			0.128 *	0.168 *	0.146 *	0.186 *		
SBT	(6)	2	3200	59	9 591	678	670			0.187	0.185	0.212	0.209		
SBR	(D)		1600	11	3 113	118	118			0.071	0.071	0.074	0.074		
EBL		2	3200	17	3 173	177	177			0.054	0.054	0.055	0.055		
EBT		2	3200	52	8 554	552	578			0.165 *	0.173 *	0.173 *	0.181 *		
EBR	(c)	1	1600	8	3 83	85	85			0.052	0.052	0.053	0.053		
WBI		2	3200	24	8 342	269	363			0.078 *	0 107 *	0.084 *	0 113 *		
WBT		2	3200	56	6 591	583	608			0.070	0.185	0.182	0.110		
WBR	(d)	1	1600	13	5 175	152	192			0.084	0.109	0.095	0.120		
							LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
				TOTAL INT	EDGECT					0.724	0.804	0.775	0.852	1	
				TOTAL INT	SCENAF	SIN CAPA	L OF SI			0.724 C	0.801 C	0.775 C	0.852		
NOTE	s.														
NOTE	0.	RTOR:	(a) 30%												
			(b) 20%												
			(c) 30%												
	Drintad	03/37/34	(a) 30%												
	Frinted:														
EXIST	ING:		< THIS COM	IPARES TO C	ONDITIC	)N (A)									

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#24017 - SEA INTERSECTI COUNT DATE TIME PERIOD N/S STREET E/W STREET CONTROL TY	<b>RS BUILD</b> DN CAPAC :: ): : /PE:	ING REMODEL PROJE ITY UTILIZATION WOF 02/22/2023 AM PEAK HOUR TOWN CENTER DRIV MAIN STREET SIGNAL	ect Rksheet Ze										REF:	02_AM
VOLUMES				SOUT	TH BOU	IND P	EAS		ND B	WES	T BOUND	P		
			0	<u> </u>	0	0		705	5	10	710	0		
(B) PROJI	ECT-ADDED	: 46 0	9 10	0	0	0	4	-6	5 40	16	-6	0		
(C) CUMU	LATIVE:	41 0	28	0	0	0	4	746	5	28	741	0		
					GE	OMETR	RICS							
		NORTH BC	DUND	SOUT	ГН ВОО	IND	EAS		ND	WES	T BOUND			
LANE GEOM	ETRICS	LF	र					L TTT	R		L TTT			
				1	TRAFFI	C SCE	NARIO	os						<u>.</u>
SCENARIO 1 SCENARIO 2 SCENARIO 3 SCENARIO 4	= EXISTIN = EXISTIN = SHORT- = SHORT-	G VOLUMES (A) G + PROJECT VOLUM TERM CUMULATIVE (( TERM CUMULATIVE +	ES(A+B) C) PROJEC	T VOLI	UMES (	<u>B+C)</u>								
	1		LE	EVEL C	OF SER	VICE	CALCI	JLATION	IS					
MOVE-	# OF	CARACITY		SCE	NARIO	VOLUM	<u>ES</u>			2	SCENARIO	V/C RATIOS	<u>8</u>	
MEN13				<u> </u>	<u> </u>	4			0.000	2	3	4		
NBL		3200	0	0	41	0/ 0			0.003	0.032	0.020	0.054		
NBR (a)	1	1600	7	15	22	30			0.004 *	0.009	0.014	0.019		
SBI	1	1600	0	0	0	0			0.000 *	0.000	0.000 *	0.000		
SBT	2	3200	0	0	0	0			0.000	0.000 *	0.000	0.000 *		
SBR (b)	1	1600	0	0	0	0			0.000	0.000	0.000	0.000		
FBI	2	3200	4	4	4	4			0.001	0.001	0.001	0.001		
EBT	2	3200	705	699	746	740			0.220 *	0.218 *	0.233 *	0.231 *		
EBR (c)	1	1600	4	36	4	36			0.003	0.023	0.003	0.023		
WBI	2	3200	19	35	28	44			0.006 *	0.011 *	0.009 *	0.014 *		
WBT	2	3200	718	712	741	735			0.224	0.223	0.232	0.230		
WBR (d)	1	1600	0	0	0	0			0.000	0.000	0.000	0.000		
						LOS	T TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
		тот	AL INTERS	SECTIO CENARI	N CAPA	LOF SE	<b>tiliza</b> Ervice	TION:	0.330 A	0.361 A	0.368 A	0.399 A		
NOTES:										1	l		1	1
Printeo	: 03/27/24													
EXISTING:		< THIS COMPARES	S TO CON	DITIO	N (A)									

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

<b>#24017</b> INTERS COUNT TIME PE N/S STF E/W STI CONTR	- SEAR BECTIO DATE: ERIOD: REET: REET: COL TYF	<b>es Build</b> N Capac	ING REMODEL CITY UTILIZATIO 02/22/2023 PM PEAK HOU TOWN CENTEL MAIN STREET SIGNAL	PROJECT N WORKSH IR R DRIVE	EET									REF:	02_PM
TRAFFIC VOLUME SUMMARY           NORTH BOUND         SOUTH BOUND         EAST BOUND         WEST BOUND															
VOLUM	ES		L	T R	L	T	R	L	T	R	L	T	R		
<ul> <li>(A) EXISTING:</li> <li>(B) PROJECT-ADDED:</li> <li>(C) CUMULATIVE:</li> </ul>			48 ): 191 70	0 41 0 40 0 53	0 0 0 0	0 0 0	0 0 0	31 0 31	821 -15 867	24 105 24	57 41 73	959 -15 999	0 0 0		
	GEOMETRICS														
LANE G	BEOME	TRICS	NOF	RTH BOUND	SOU	TH BOL	JND	EAS	ST BOUI	ND R	WES	T BOUND			
						TRAFFI	IC SCE	NARIO	DS						
SCENAI SCENAI SCENAI SCENAI	RIO 1 = RIO 2 = RIO 3 = RIO 4 =	EXISTIN EXISTIN SHORT- SHORT-	IG VOLUMES (A IG + PROJECT \ TERM CUMULA TERM CUMULA	) /OLUMES(A .TIVE (C) .TIVE + PRO	+B) DJECT VOL	UMES (	(B+C)								
					LEVEL	OF SER		CALCU	JLATIO	NS					
MOVE-			CABACITY	/ I	1 3	ENARIO	VOLUM	<u>ES</u>		1	2	SCENARIO Y		<u> </u>	
NBI		LANES	1600		48 230	3 70	261			0.030 *	∠ 0.149. *	0 044 *	• 0 163 *		
NBT		2	3200		0 0	0	0			0.000	0.000	0.000	0.000		
NBR	(a)	1	1600		33 65	42	74			0.021	0.041	0.026	0.046		
SBL		1	1600		0 0	0	0			0.000	0.000	0.000	0.000		
SBT		2	3200		0 0	0	0			0.000 *	0.000 *	0.000 *	0.000 *		
SBR	(b)	1	1600		0 0	0	0			0.000	0.000	0.000	0.000		
EBL		2	3200		31 31	31	31			0.010 *	0.010 *	0.010 *	0.010 *		
EBT		2	3200	8	21 806	867	852			0.257	0.252	0.271	0.266		
EBR	(c)	1	1600		19 103	19	103			0.012	0.064	0.012	0.064		
WBL		2	3200		57 98	73	114			0.018	0.031	0.023	0.036		
WBT		2	3200	9	59 944	999	984			0.300 *	0.295 *	0.312 *	0.308 *		
WBR	(d)	1	1600		0 0	0	0			0.000	0.000	0.000	0.000		
							LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
				TOTAL IN	TERSECTIC SCENAR	IO LEVE	ACITY U	TILIZA ERVICE	TION:	0.440 A	0.554 A	0.466 A	0.581 A		
NOTES	:									Į	Į	1	I	I	
	Printed:	03/27/24													

EXISTING: <---- THIS COMPARES TO CONDITION (A) SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B) SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#24017 - SEAF INTERSECTIO COUNT DATE: TIME PERIOD: N/S STREET: E/W STREET: CONTROL TYP	RS BUILD N CAPAC	DING REMODEL PF CITY UTILIZATION 01/18/2023 AM PEAK HOUR MILLER STREET MAIN STREET SIGNAL	ROJECT WORKSHEET	Г		01.1.7							REF:	03_AM
		NORT		SOU	TH BOI		E SUN			W/ES				
VOLUMES		L	T R	L	Т	R	L	T	R	L	T	R		
(A) EXISTIN	IG:	83	173 145	74	231	65	48	567	50	177	608	61		
(B) PROJEC	CT-ADDED	): <b>0</b>	1 4	0	1	2	0	4	0	9	8	0		
(C) CUMUL	ATIVE:	87	202 158	76	242	66	51	623	58	182	652	61		
					GE	OMETI								
		NODT		8011			<u> </u>							
LANE GEOME	TRICS	NORT	T TR	500		R	EAS	L TT	R	VVES	L T TR			
					TRAFF	IC SCE	NARIO	os						
SCENARIO 1 = SCENARIO 2 = SCENARIO 3 = SCENARIO 4 =	SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B) SCENARIO 3 = SHORT-TERM CUMULATIVE (C) SCENARIO 4 = SHORT-TERM CUMULATIVE + PROJECT VOLUMES (B+C)													
	1		L	EVEL (	OF SER	<b>VICE</b>	CALCU	JLATIO	NS					
MOVE-	MOVE- # OF <u>SCENARIO VOLUMES</u> <u>SCENARIO V/C RATIOS</u>													
MENTS	LANES		1	2	3	4			1	2	3	4		
NBL	1	1600	83	83	87	87			0.052	0.052	0.054	0.054		
NBR (a)		3200	173	174 149	202	203			0.099 ^	0.101 *	0.113 *	0.114 *		
(0)														
SBL	1	1600	74	74	76	76			0.046 *	0.046 *	0.048 *	0.048 *		
SBT	2	3200	231	232	242	243			0.093	0.093	0.096	0.097		
SBR (b)	0	0	65	67	66	68			-	-	-	-		
EBL	1	1600	48	48	51	51			0.030	0.030	0.032	0.032		
EBT	2	3200	567	571	623	627			0.177 *	0.178 *	0.195 *	0.196 *		
EBR (c)	1	1600	35	35	41	41			0.022	0.022	0.026	0.026		
W/BI	1	1600	177	186	192	101			0 1 1 1 *	0.116 *	0.111 *	0.110 *		
WBT	2	3200	608	616	652	660			0.209	0.212	0.223	0.225		
WBR (d)	0	0	61	61	61	61			-	-	-	-		
	1	1	- I			LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
					ON CAPA	ACITY U	TILIZA	TION:	0.533 A	0.541 A	0.570 A	0.577 A		
NOTES														
NOTES.	RTOR:	(a) 0% (b) 0% (c) 30% (d) 0%												
Printed:	03/27/24	(0) 070												

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#24017 - SEAF INTERSECTIO COUNT DATE: TIME PERIOD: N/S STREET: E/W STREET: CONTROL TY	RS BUILE N CAPAC	DING REMODEL P CITY UTILIZATION 01/18/2023 PM PEAK HOUR MILLER STREET MAIN STREET SIGNAL	ROJECT I WORKSHEE R T	Т									REF:	03_PM
		NOR		SOU	TH BOU	JND	EAS		ND	. WES	ST BOUND	_		
VOLUMES		L	I R	L		R	L	I	R	L	I	R		
(A) EXISTIN	NG:	144	358 230	93	320	74	114	617 22	122	176	759	81		
(C) CUMUL	ATIVE:	146	4 22 375 236	93	4 346	4 77	3 117	663	0 127	22 184	22 806	81		
( )														
GEOMETRICS														
		NOR	TH BOUND	SOU	TH BOU	JND	EAS	ST BOU	ND	WES	ST BOUND			
LANE GEOME	TRICS	l	LTTR		LTT	R		L TT	R		L T TR			
					TRAFF	IC SCI	ENARIC	DS						
SCENARIO 1 = SCENARIO 2 = SCENARIO 3 = SCENARIO 4 =	= EXISTIN = EXISTIN = SHORT = SHORT	IG VOLUMES (A) IG + PROJECT V( -TERM CUMULAT -TERM CUMULAT	DLUMES(A+B) TVE (C) TVE + PROJEC	CT VOL	UMES	(B+C)								
		•	L	EVEL	OF SER	VICE	CALCU	JLATIO	NS					
MOVE-	# OF		1.	SCE	ENARIO	VOLUN	<u>/IES</u>		1.	5	SCENARIO	V/C RATIO	<u>8</u>	
MENTS	LANES		1	2	3	4			1	2	3	4	1	
NBL		1600	144	144	146 275	146			0.090	0.090	0.091	0.091		
NBR (a)	0	0	230	252	236	258			-	-	-	-		
SBL	1	1600	93	93	93	93			0.058 *	0.058 *	0.058 *	0.058 *		
SBT	2	3200	320	324	346	350			0.123	0.126	0.132	0.135		
			14	70		01			-	-				
EBL	1	1600	114	117	117	120			0.071 *	0.073 *	0.073 *	0.075 *		
EBT	2	3200	617	639	663	685			0.193	0.200	0.207	0.214		
EBR (c)	1	1600	85	85	89	89			0.053	0.053	0.056	0.056		
WBL	1	1600	176	198	184	206			0.110	0.124	0.115	0.129		
WBT	2	3200	759	781	806	828			0.263 *	0.269 *	0.277 *	0.284 *		
WBR <i>(d)</i>	0	0	81	81	81	81			-	-	-	-		
						LO	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
			TOTAL INTER	RSECTIC	ON CAPA	ACITYI	JTILIZAT	TION:	0.676	0.692	0.699	0.716		
			S	CENAR	IO LEVE	LOFS	ERVICE		В	В	В	c		
NOTES:									1		1	1	1	
	RTOR:	(a) 0% (b) 0% (c) 30% (d) 0%												
Printed:	03/27/24	(0) 0 /0												

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)
		H	HCS <sup>-</sup>	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	н						Inters	ection			ELIZA	BETH ST	/MAIN S	ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARIA	4			
Date Performed	04/02	2/2024					East/	West Stre	eet		MAIN	I ST				
Analysis Year	2024						North	n/South S	Street		ELIZA	BETH ST	-			
Time Analyzed	AM P	eak ho	UR				Peak	Hour Fac	ctor		0.92					
Intersection Orientation	East-	West					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	EXIST	ING														
Lanes         Vehicle Volumes and Adj         Approach         Movement         Priority	USTRE	nts Eastl	pound T 2			↔ ↔ or Street: Ea Westl	b b 4			North	bound T 8	R 9		South L 10	bound T 11	R 12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	1	L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	14	683	3	0	14	769	20		4	1	17		9	3	23
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked	1															
Percent Grade (%)											0				) )	
Right Turn Channelized	1															
Median Type   Storage	1			Left	Only								2			
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)	1	4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)	1	2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of S	ervice			-	<u> </u>	<u> </u>			<u> </u>	-		<u> </u>		
Flow Rate, v (veh/h)	1	15				15					24				38	
Capacity, c (veh/h)		772				852					421				329	
v/c Ratio	1	0.02				0.02					0.06				0.12	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1				0.1					0.2				0.4	
Control Delay (s/veh)	1	9.8				9.3					14.1				17.4	
Level of Service (LOS)	1	Α				A					В				С	
Approach Delay (s/veh)	1	C	.2			0	.2			- 14	4.1			17	7.4	
Approach LOS			A				A				В			(	2	

		H	HCS <sup>-</sup>	Гwo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	JН						Inters	ection			ELIZA	BETH ST	/MAIN S	ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARI	4			
Date Performed	04/02	2/2024					East/	West Stre	eet		MAIN	I ST				
Analysis Year	2024						North	n/South S	Street		ELIZA	BETH ST	-			
Time Analyzed	AM P	PEAK HO	UR				Peak	Hour Fac	ctor		0.92					
Intersection Orientation	East-	West					Analy	sis Time	Period	(hrs)	0.25					
Project Description	EXIST	TING + P	ROJECT								•					
Lanes																
				J 4 1 7 4 1 7		م م Street: Ea	↑ ┾ ↑									
Vehicle Volumes and Adju	Istments															
Approach		Eastk	oound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	1	L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	15	690	3	0	14	784	20		4	1	17		9	3	25
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked	1															
Percent Grade (%)	1										0			(	)	
Right Turn Channelized	1															
Median Type   Storage	1			Left	Only								2			
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)	1	4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)	1	2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	1	16				15					24				40	
Capacity, c (veh/h)	i	762				846					415				329	
v/c Ratio	İ	0.02				0.02					0.06				0.12	
95% Queue Length, Q <sub>95</sub> (veh)	İ	0.1				0.1					0.2				0.4	
Control Delay (s/veh)	i	9.8				9.3					14.2				17.5	
Level of Service (LOS)	i	А				А					В				С	
Approach Delay (s/veh)	i	C	).2			C	).2			- 14	4.2			17	7.5	
Approach LOS	1	A     A     A     B     C       0.2     0.2     14.2     17.5       A     A     B     C														

		ł	HCS <sup>-</sup>	Two-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inform	natio	n						
Analyst	JH						Inters	ection			ELIZA	BETH ST	/main s	ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARI	Ą			
Date Performed	04/02	2/2024					East/	West Stre	eet			I ST				
Analysis Year	2024	·					North	n/South S	Street		ELIZA	BETH ST	Г			
Time Analyzed	AM P	PEAK HO	UR				Peak	Hour Fac	ctor		0.92					
Intersection Orientation	East-	West					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	СЛМ	ULATIVE									1					
				J 4 1 4 4 4 0		↓ <u>↓</u>										
Vehicle Volumes and Ad	ustme	nts			Maj	or Street: Ea	st-West									
Approach		Eastk	oound			West	bound			North	bound		Í	South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	14	754	3	0	14	818	20		4	1	17		9	3	23
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0			(	0	
Right Turn Channelized																
Median Type   Storage				Left	Only								2			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of S	ervice	,												-
Flow Rate, v (veh/h)		15				15					24				38	
Capacity, c (veh/h)		737				796					382				297	
v/c Ratio		0.02				0.02					0.06				0.13	
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.1					0.2				0.4	
Control Delay (s/veh)		10.0				9.6					15.1				18.9	
Level of Service (LOS)		А				A					С				С	
Approach Delay (s/veh)		C	).2			0	.2			1	5.1			18	3.9	
Approach LOS			A				A				С			(	С	

		ŀ	HCS <sup>-</sup>	Two-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						_
Analyst	ЛН						Inters	ection			ELIZA	BETH ST	/MAIN S	ST ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARI	4			
Date Performed	04/02	2/2024					East/	West Str	eet		MAIN	I ST				
Analysis Year	2024						North	n/South :	Street		ELIZA	BETH ST	-			
Time Analyzed	AM F	PEAK HO	UR				Peak	Hour Fac	ctor		0.92					
Intersection Orientation	East-	West					Analy	sis Time	Period (	(hrs)	0.25					
Project Description	СЛМ	ULATIVE	+ PROJ	ECT							1					
Lanes				*		* <u>1 1 1</u>	<u>+                                    </u>									
				14 1 7 4 P 1	h Maj	¢ or Street: Ea	↑ Դ ↑ ast-West	4 1 4 4 4 4 4 4								
Vehicle Volumes and Ad	justme	stments														
Approach		Eastk	bound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	15	761	3	0	14	833	20		4	1	17		9	3	25
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0			(	0	
Right Turn Channelized																
Median Type   Storage				Left	Only								2			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of S	ervice			<u> </u>	<u> </u>		<u> </u>	<u> </u>		<u> </u>	<u> </u>			
Flow Bate v (veh/h)	1	16			<u> </u>	15			<u> </u>		24	<u> </u>			40	
Capacity c (yeb/b)		727				791					375				298	
		0.02				0.02					0.06				0.14	
95% Queue Longth Q (uch)		0.02				0.02					0.00				0.14	
Control Dolay (chich)		10.1				0.1					15.2				10.0	
	-	IU.I				9.0					15.2				19.0	
						A										
Approach Delay (s/veh)		C	0.2			C	0.2			1	o.2			19	1.U	
Approach LOS			A				A				L			(	-	

AWD = 15.1 sec. (LOS C)

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		ŀ	HCS <sup>-</sup>	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	ЛН						Inters	ection			ELIZA	BETH ST	/MAIN S	ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARI	Ą			
Date Performed	04/02	2/2024					East/	West Stre	eet		MAIN	I ST				
Analysis Year	2024	-					North	n/South S	Street		ELIZA	BETH ST	Г			
Time Analyzed	PM P	EAK HO	UR				Peak	Hour Fac	ctor		0.94					
Intersection Orientation	East-	West					Analy	sis Time	Period	(hrs)	0.25					
Project Description	EXIST	ING									•					
Lanes																
				$J \neq \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow$	ĥ.	ф <b>•</b> Т	t ;; 7	1 1 1 4 4 1 1 1								
Vehicle Volumes and Adju	ustments															
Approach		Eastk	oound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	1	L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	29	1048	7	0	13	954	34		9	3	43		10	2	51
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked	1															
Percent Grade (%)	1										0			(	)	
Right Turn Channelized	1															
Median Type   Storage	1			Left	Only								2			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)	1	4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)	1	2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of S	ervice					• •								
Flow Rate, v (veh/h)	1	31				14					59				67	
Capacity, c (veh/h)	j	652				612					242				300	
v/c Ratio	i	0.05				0.02					0.24				0.22	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1				0.1					0.9				0.8	
Control Delay (s/veh)	1	10.8				11.0					24.6				20.4	
Level of Service (LOS)		В				В					С				С	
Approach Delay (s/veh)	1	C	).3			C	0.1			24	4.6			20	).4	
Approach LOS	i		A				A				С			(	2	

		ŀ	HCS <sup>-</sup>	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	н.						Inters	ection			ELIZA	BETH ST	/MAIN S	ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARIA	4			
Date Performed	04/02	2/2024					East/	West Stre	eet		MAIN	I ST				
Analysis Year	2024	-					North	n/South S	Street		ELIZA	BETH ST	-			
Time Analyzed	PM P	EAK HO	UR				Peak	Hour Fac	ctor		0.94					
Intersection Orientation	East-'	West					Analy	sis Time	Period	(hrs)	0.25					
Project Description	EXIST	'ING + P	ROJECT								1					
Lanes																
				2 4 4 5 4 6 6 2 4 1 5 6		منبع or Street: Ea	t t T	1 1 4 4 7 1 4 1								
Vehicle Volumes and Adj	ustme	stments														
Approach	1	Eastk	ound		ir 👘	West	bound			North	bound		í –	South	bound	
Movement		L L	Т	R	U	L	Т	R	U	L	Т	R		L	Т	R
Priority	1 10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	1	L	Т	TR		L	т	TR			LTR				LTR	
Volume (veh/h)	0	35	1086	7	0	13	992	34		9	3	43		10	2	57
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked	1															
Percent Grade (%)	1										0				)	
Right Turn Channelized	1															
Median Type   Storage	1			Left	Only								2			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)	]	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	d Leve	l of S	ervice			<u> </u>		<u> </u>			<u> </u>		<u> </u>			
Flow Rate, v (veh/h)	1	37				14					59				73	
Capacity, c (veh/h)	1	629				591					221				289	
v/c Ratio	1	0.06				0.02					0.26				0.25	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.2				0.1					1.0				1.0	
Control Delay (s/veh)	1	11.1				11.2					27.0				21.6	
Level of Service (LOS)	1	В				В					D				С	
Approach Delay (s/veh)	1	C	).3			0	.1			2	7.0			2	1.6	
Approach LOS	1		A				A				D				2	
	1	_	_					_		_			1	_	_	

		ł	HCS <sup>-</sup>	Гwo-	Way	Stop	o-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	Л						Inters	ection			ELIZA	BETH ST	/MAIN S	ST		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARIA	4			
Date Performed	04/0	2/2024					East/	West Stre	eet			I ST				
Analysis Year	2024						North	/South	Street			BETH ST	-			
Time Analyzed	PM P	EAK HO	UR				Peak	Hour Fac	ctor		0.94					
Intersection Orientation	East-	West	-				Analy	sis Time	Period	(hrs)	0.25					
Project Description	СОМ	ULATIVE								- /						
Lanes																
				2 4 1 7 4 P	۲ Maj	منبع or Street: Ea	<b>↑ ネ ア</b> ıst-West	1 1 4 4 7 1 1 1 4 4 4 7 1								
Vehicle Volumes and Ad	justme	ustments														
Approach		Eastl	oound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	1	L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	29	1100	7	0	13	1009	34		9	3	43		10	2	51
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked	1															
Percent Grade (%)	1										0				0	
Right Turn Channelized	1															
Median Type   Storage	1			Left	Only								2			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)	7	4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)	7	2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of S	ervice		<u> </u>	I	<u> </u>	<u> </u>	<u> </u>		I	<u> </u>	<u> </u>	<u> </u>	I	<u> </u>
		21				14					E0				67	
Flow Rate, V (Verl/II)	╡──	610				E02					210				275	
		0.05				0.02					0.27				0.24	
05% Quare Longth Q (up)		0.05				0.02					1.0				0.24	
Control Dolay (chich)		0.2				0.1					1.0				0.9	
		,   				II.3					21.3				22.3	
Level of Service (LUS)		В	12			В	1									
Approach Delay (s/veh)	╡──	(	.3			0	A. I			2	1.3			2.	2.3 C	
Approach LOS	_		A				A				U				<b>_</b>	

		ł	HCS 1	Гwo-	Way	Stop	-Cor	ntrol	Repo	ort						
General Information	_	_	_	_	_	_	Site	Inforr	natio	n	_	_	_	_	_	_
Analyst	Н						Inters	ection			ELIZA	BETH ST	/MAIN S	ST		
Agency/Co.	ATE						Jurisd	liction			SANT	A MARI	4			
Date Performed	04/02	2/2024					East/	Nest Stre	eet		MAIN	I ST				
Analysis Year	2024						North	/South :	Street		ELIZA	BETH ST	-			
Time Analyzed	PM P	EAK HO	UR				Peak	Hour Fac	ctor		0.94					
Intersection Orientation	East-	West					Analy	sis Time	Period	(hrs)	0.25					
Project Description	СОМ	ULATIVE	+ PROJE	CT							1					
Lanes																
				$J \neq \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow$	n 4	*	1 4 1	1 1 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4								
Vehicle Volumes and Adju	ustme	nts			Maj	or Street: Ea	st-West									
Approach		Eastk	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	1	L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	35	1138	7	0	13	1047	34		9	3	43		10	2	57
Percent Heavy Vehicles (%)	3	3			3	3				3	3	3		3	3	3
Proportion Time Blocked	1															
Percent Grade (%)	1										0				0	
Right Turn Channelized	1															
Median Type   Storage	1			Left	Only								2			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.16				4.16				7.56	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	l Leve	l of S	ervice													
Flow Rate, v (veh/h)	1	37				14					59				73	
Capacity, c (veh/h)		598				563					199				265	
v/c Ratio	1	0.06				0.02					0.29				0.28	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.2				0.1					1.2				1.1	
Control Delay (s/veh)	1	11.4				11.6					30.4				23.7	
Level of Service (LOS)	1	В				В					D				C	
Approach Delay (s/veh)		(	.3			0	.1			3(	).4			2:	3.7	
Approach LOS	1		A				A				D			(	C	

		_		_					_							
			ICS -	Гwo-'	Way	Stop	-Cor	ntrol	Repo	ort						
General Information							Site	Inforr	natio	n						
Analyst	н						Inters	ection			MILLE	ER ST/CH	IURCH S	т		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARIA	ł			
Date Performed	04/02	2/2024					East/	West Stre	eet		CHUF	RCH ST				
Analysis Year	2024						North	n/South S	Street		MILLE	ER ST				
Time Analyzed	AM P	EAK HO	JR				Peak	Hour Fac	ctor		0.86					
Intersection Orientation	North	n-South					Analy	sis Time	Period (	hrs)	0.25					
Project Description	EXIST	ING														
Lanes																
Vehicle Volumes and Adju	ıstme	nts			ብ ጉ <sub>Major</sub>	↑↑ ↑ ♥ Ƴ Street: Nor	┝ ↑ ႃ∕ ſ th-South	7 4 4 7								
Approach	1	Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	i	10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes	i –	0	1	1		0	1	0	0	1	2	0	0	0	2	1
Configuration	i –	LT		R			LTR			L	Т	TR			т	R
Volume (veh/h)	i	7	0	5		14	1	23	0	44	346	54			421	32
Percent Heavy Vehicles (%)	i	3	3	3		3	3	3	3	3						
Proportion Time Blocked	1															
Percent Grade (%)	1		0	-			0					-			-	
Right Turn Channelized	1	Ν	lo											Ν	lo	
Median Type   Storage	1			Left	Only								1			
Critical and Follow-up He	adwa	ys														

Critical and Follow-up fleadw	lays

Critical and Follow-up He	adwa	ys										
Base Critical Headway (sec)		7.5	6.5	6.9	7.5	6.5	6.9	4.1				
Critical Headway (sec)		7.56	6.56	6.96	7.56	6.56	6.96	4.16				
Base Follow-Up Headway (sec)		3.5	4.0	3.3	3.5	4.0	3.3	2.2				
Follow-Up Headway (sec)		3.53	4.03	3.33	3.53	4.03	3.33	2.23				
Delay, Queue Length, and	Leve	l of Se	ervice									
Flow Rate, v (veh/h)		8		6		44		51				
Capacity, c (veh/h)		363		753		720		1029				
v/c Ratio		0.02		0.01		0.06		0.05				
95% Queue Length, Q <sub>95</sub> (veh)		0.1		0.0		0.2		0.2				
Control Delay (s/veh)		15.1		9.8		10.3		8.7				
Level of Service (LOS)		C		А		В		А				
Approach Delay (s/veh)		12	2.9		1(	).3		0	.9			
Approach LOS			В			3			Ą		 	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	JH	Intersection	MILLER ST/CHURCH ST
Agency/Co.	ATE	Jurisdiction	SANTA MARIA
Date Performed	04/02/2024	East/West Street	CHURCH ST
Analysis Year	2024	North/South Street	MILLER ST
Time Analyzed	AM PEAK HOUR	Peak Hour Factor	0.86
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	EXISTING + PROJECT		
Lanes			



Vehicle Volumes and Adjustments           Approach         Eastbound         Westbound         Northbound         Southbound																	
Approach		Eastb	ound			West	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	1		0	1	0	0	1	2	0	0	0	2	1	
Configuration		LT		R			LTR			L	Т	TR			Т	R	
Volume (veh/h)		16	1	16		14	3	23	0	61	342	54			417	46	
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3							
Proportion Time Blocked																	
Percent Grade (%)		(	C			(	C										
Right Turn Channelized		N	lo											N	lo		
Median Type   Storage	Left Only												1				
Critical and Follow-up He	adwa	idways															
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1							
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2							
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23							
Delay, Queue Length, and	Leve	l of Se	ervice														
Flow Rate, v (veh/h)		20		19			47			71							
Capacity, c (veh/h)		331		755			617			1019							
v/c Ratio		0.06		0.02			0.08			0.07							
95% Queue Length, Q <sub>95</sub> (veh)		0.2		0.1			0.2			0.2							
Control Delay (s/veh)		16.6		9.9			11.3			8.8							
Level of Service (LOS)		C A					В			А							
Approach Delay (s/veh)	13.3 11.3								1.2								
Approach LOS		B B								1	4						

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	H	Intersection	MILLER ST/CHURCH ST
Agency/Co.	ATE	Jurisdiction	SANTA MARIA
Date Performed	04/02/2024	East/West Street	CHURCH ST
Analysis Year	2024	North/South Street	MILLER ST
Time Analyzed	AM PEAK HOUR	Peak Hour Factor	0.86
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	CUMULATIVE		
Lanes			



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			West	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	l I	10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes	1	0	1	1		0	1	0	0	1	2	0	0	0	2	1
Configuration	1	LT		R			LTR			L	Т	TR			Т	R
Volume (veh/h)	1	7	0	5		14	1	23	0	44	392	54			445	32
Percent Heavy Vehicles (%)	1	3	3	3		3	3	3	3	3						
Proportion Time Blocked	1															
Percent Grade (%)	1	(	0			(	)									
Right Turn Channelized	1	N	lo											N	lo	
Median Type   Storage	Left Only												1			
Critical and Follow-up He	adwa	lways														
Base Critical Headway (sec)	1	7.5	6.5	6.9		7.5	6.5	6.9		4.1						
Critical Headway (sec)	1	7.56	6.56	6.96		7.56	6.56	6.96		4.16						
Base Follow-Up Headway (sec)	1	3.5	4.0	3.3		3.5	4.0	3.3		2.2						
Follow-Up Headway (sec)	1	3.53	4.03	3.33		3.53	4.03	3.33		2.23						
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	8		6			44			51						
Capacity, c (veh/h)	i i	344		737			673			1005						
v/c Ratio	l –	0.02		0.01			0.07			0.05						
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1		0.0			0.2			0.2						
Control Delay (s/veh)	1	15.7		9.9			10.7			8.8						
Level of Service (LOS)			В			А										
Approach Delay (s/veh)	13.3 10.7								0.8							
Approach LOS	1	I	В			I	3			/	4					

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	H	Intersection	MILLER ST/CHURCH ST
Agency/Co.	ATE	Jurisdiction	SANTA MARIA
Date Performed	04/02/2024	East/West Street	CHURCH ST
Analysis Year	2024	North/South Street	MILLER ST
Time Analyzed	AM PEAK HOUR	Peak Hour Factor	0.86
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	CUMULATIVE + PROJECT		
Lanes			



Vehicle Volumes and Adju	Volumes and Adjustments           n         Eastbound         Westbound         Northbound         Southbound															
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1	10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes	1	0	1	1		0	1	0	0	1	2	0	0	0	2	1
Configuration	1	LT		R			LTR			L	Т	TR			Т	R
Volume (veh/h)	1	16	1	16		14	3	23	0	61	388	54			441	46
Percent Heavy Vehicles (%)	1	3	3	3		3	3	3	3	3						
Proportion Time Blocked	1															
Percent Grade (%)		(	0			(	C									
Right Turn Channelized	1	N	lo											Ν	lo	
Median Type   Storage	Left Only												1			
Critical and Follow-up He	adwa	adways														
Base Critical Headway (sec)	1	7.5	6.5	6.9		7.5	6.5	6.9		4.1						
Critical Headway (sec)	1	7.56	6.56	6.96		7.56	6.56	6.96		4.16						
Base Follow-Up Headway (sec)	1	3.5	4.0	3.3		3.5	4.0	3.3		2.2						
Follow-Up Headway (sec)	1	3.53	4.03	3.33		3.53	4.03	3.33		2.23						
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	20		19			47			71						
Capacity, c (veh/h)	1	311		740			571			995						
v/c Ratio	1	0.06		0.03			0.08			0.07						
95% Queue Length, Q <sub>95</sub> (veh)	1	0.2		0.1			0.3			0.2						
Control Delay (s/veh)		17.4		10.0			11.9			8.9						
Level of Service (LOS)		С		А			В			А						
Approach Delay (s/veh)	13.8 11.9									1	.1					
Approach LOS	B B									A						

		ŀ	ICS 1	Гwo-`	Wav	Stop	-Cor	ntrol	Repo	ort						
General Information	_	-	-	_		-	Sito	Inform	natio	n	-	-	_	_	_	
	-						pite		natio							
Analyst	J.H						Inters	ection			MILLE	R ST/CF	HURCH S	T		
Agency/Co.	ATE						Jurisc	liction			SANT	A MARIA	4			
Date Performed	04/02	2/2024					East/	Nest Stre	eet		CHUF	RCH ST				
Analysis Year	2024						North	/South S	Street		MILLE	ER ST				
Time Analyzed	PM P	EAK HO	JR				Peak	Hour Fac	ctor		0.93					
Intersection Orientation	North	n-South					Analy	sis Time	Period (	hrs)	0.25					
Project Description	EXIST	ING														
Lanes																
					ብ ጉ <sub>Majo</sub>	기 ↑ ↑ ↑ r Street: Noi	┝ ↑ ┣ ሾ th-South	+ + + + + + + + + + + + + + + + + + +								
	ustme	nts														
Approach		Eastk	ound			West	bound			North	bound			South	bound	
Movement	U		T	R	U	L	Т	R	U	L	Т	R	U		Т	R
Priority	4	10	11	12		7	8	9	10	1	2	3	40	4	5	6
Number of Lanes	<u> </u>	0	1	1		0	1	0	0	1	2	0	0	0	2	1
Configuration		LT		R			LTR			L	Т	TR			Т	R
Volume (veh/h)	]	34	11	29		9	6	51	0	34	733	32			455	105
Percent Heavy Vehicles (%)	]	3	3	3		3	3	3	3	3						
Proportion Time Blocked	]															
Percent Grade (%)	1		0				0									
Right Turn Channelized	1	Ν	10											Ν	10	
Median Type   Storage	1			Left	Only								1			
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)	7.5 6.5 6.9 7.5 6.5 6.9 4.1															
Critical Headway (sec)	1	7.56	6.56	6.96		7.56	6.56	6.96		4.16						
Base Follow-Up Headway (sec)	j	3.5	4.0	3.3		3.5	4.0	3.3		2.2						
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23						
Delay, Queue Length, and	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	1	48		31			71			37						
Canacity c (veh/h)		228		752			525			964						

0.21

0.8

25.0

С

19.1

С

0.04

0.1

10.0

А

12.9

В

0.14

0.5

12.9

В

0.04

0.1

8.9

А

0.4

А

95% Queue Length, Q<sub>95</sub> (veh)

Control Delay (s/veh)

Level of Service (LOS)

Approach LOS

Approach Delay (s/veh)

v/c Ratio

HCS Two-Way Stop	-Control Report											
	Site Information											
JH	Intersection	MILLER ST/CHURCH ST										
ATE	Jurisdiction	SANTA MARIA										
04/02/2024	East/West Street	CHURCH ST										
2024	North/South Street	MILLER ST										
PM PEAK HOUR	Peak Hour Factor	0.93										
North-South	Analysis Time Period (hrs)	0.25										
EXISTING + PROJECT												
	HCS Two-Way Stop	HCS Two-Way Stop-Control Report         Site Information         JH       Intersection         ATE       Jurisdiction         04/02/2024       East/West Street         2024       North/South Street         PM PEAK HOUR       Peak Hour Factor         North-South       Analysis Time Period (hrs)         EXISTING + PROJECT       Image: Colspan="2">Image: Colspan="2"         Image: Colspan="2">Image: Colspan="2"										



Vehicle Volumes and Adju	istme	nts														
Approach		Eastb	ound			West	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	1		0	1	0	0	1	2	0	0	0	2	1
Configuration		LT		R			LTR			L	Т	TR			Т	R
Volume (veh/h)		71	17	72		9	12	51	0	78	722	32			444	142
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3						
Proportion Time Blocked																
Percent Grade (%)		(	)			(	)									
Right Turn Channelized		N	0											N	0	
Median Type   Storage	Left Only												1			
Critical and Follow-up He	adways															
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1						
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16						
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2						
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23						
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		95		77			77			84						
Capacity, c (veh/h)		200		759			368			941						
v/c Ratio		0.47		0.10			0.21			0.09						
95% Queue Length, Q <sub>95</sub> (veh)		2.3		0.3			0.8			0.3						
Control Delay (s/veh)		38.2		10.3			17.4			9.2						
Level of Service (LOS)		E		В			С			А						
Approach Delay (s/veh)	25.7 17.4									0	.9					
Approach LOS	D C							A								

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	н	Intersection	MILLER ST/CHURCH ST
Agency/Co.	ATE	Jurisdiction	SANTA MARIA
Date Performed	04/02/2024	East/West Street	CHURCH ST
Analysis Year	2024	North/South Street	MILLER ST
Time Analyzed	PM PEAK HOUR	Peak Hour Factor	0.93
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	CUMULATIVE	·	
Lanes			
		9 년 4 3 4 수 7	

う ↑ ↑ <u>ううす や マ ↑ ♪ ↑</u> Major Street: North-South

Vehicle Volumes and Adju	istme	nts																
Approach		Eastb	ound			West	bound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	1	1		0	1	0	0	1	2	0	0	0	2	1		
Configuration		LT		R			LTR			L	Т	TR			Т	R		
Volume (veh/h)		34	11	29		9	6	51	0	34	758	32			494	105		
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3								
Proportion Time Blocked																		
Percent Grade (%)		(	)			(	2											
Right Turn Channelized		N	0											N	0			
Median Type   Storage		Left Only											1					
Critical and Follow-up He	adwa	dways																
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1								
Critical Headway (sec)		7.56	6.56	6.96		7.56	6.56	6.96		4.16								
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2								
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23								
Delay, Queue Length, and	Leve	l of Se	ervice															
Flow Rate, v (veh/h)		48		31			71			37								
Capacity, c (veh/h)		212		730			499			930								
v/c Ratio		0.23		0.04			0.14			0.04								
95% Queue Length, Q <sub>95</sub> (veh)		0.9		0.1			0.5			0.1								
Control Delay (s/veh)		26.9		10.2			13.4			9.0								
Level of Service (LOS)		D B					В			А								
Approach Delay (s/veh)	20.3 13.4									0	.4							
Approach LOS	C B							A										

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	H	Intersection	MILLER ST/CHURCH ST
Agency/Co.	ATE	Jurisdiction	SANTA MARIA
Date Performed	04/02/2024	East/West Street	CHURCH ST
Analysis Year	2024	North/South Street	MILLER ST
Time Analyzed	PM PEAK HOUR	Peak Hour Factor	0.93
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	CUMULATIVE + PROJECT		
Lanes			



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1	10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes	1	0	1	1		0	1	0	0	1	2	0	0	0	2	1
Configuration	1	LT		R			LTR			L	Т	TR			Т	R
Volume (veh/h)	1	71	17	72		9	12	51	0	78	747	32			483	142
Percent Heavy Vehicles (%)	1	3	3	3		3	3	3	3	3						
Proportion Time Blocked	1															
Percent Grade (%)	1	(	0			(	C									
Right Turn Channelized	1	Ν	lo											Ν	lo	
Median Type   Storage	1	Left Only											1			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)	1	7.5	6.5	6.9		7.5	6.5	6.9		4.1						
Critical Headway (sec)	1	7.56	6.56	6.96		7.56	6.56	6.96		4.16						
Base Follow-Up Headway (sec)	l I	3.5	4.0	3.3		3.5	4.0	3.3		2.2						
Follow-Up Headway (sec)	1	3.53	4.03	3.33		3.53	4.03	3.33		2.23						
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	95		77			77			84						
Capacity, c (veh/h)	l	186		736			340			908						
v/c Ratio	1	0.51		0.11			0.23			0.09						
95% Queue Length, Q <sub>95</sub> (veh)	i i	2.5		0.4			0.9			0.3						
Control Delay (s/veh)		43.0		10.5			18.7			9.4						
Level of Service (LOS)	1	E		В			С			А						
Approach Delay (s/veh)		28.4 18.7 0.9														
Approach LOS	1	[	C			(	C			ļ	4					

#2401 INTER COUN TIME N/S S E/W S CONT	IT - SEAF RSECTIO NT DATE: PERIOD: TREET: STREET: IROL TYI	<b>RS BUILD</b> IN CAPAC	ING REMODEL PRO CITY UTILIZATION W 01/18/2023 AM PEAK HOUR BROADWAY COOK STREET SIGNAL	DJECT ORKSHEET	г									REF:	06_AM
					TRA	FFIC V	OLUME	E SUN	IMARY						
VOU			NORTH		SOU	TH BOU	JND	EAST BOUND			WES	T BOUND	B		
VOLU	INES	10		<u> </u>	L	1	ĸ		1	<u> </u>		1	<u> </u>		
(A) (B)	PROJE	NG: CT-ADDEE	): 0 2	52 /9 7 0	44 0	697 16	64 3	92 5	135 0	66 0	52 0	84 0	24		
(C)	CUMUL	ATIVE:	67 6	34 83	55	765	68	94	136	66	60	86	39		
	GEOMETRICS														
			NORTH	BOUND	SOUTH BOUND			EAST BOUND			WES	T BOUND			
LANE	GEOME	TRICS	L	TT R		LTT	R		LTTF	२		L T TR			
						TRAFF	IC SCE	NARIO	DS						
SCEN SCEN SCEN SCEN	SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B) SCENARIO 3 = SHORT-TERM CUMULATIVE (C) SCENARIO 4 = SHORT-TERM CUMULATIVE + PROJECT VOLUMES (B+C)														
				L	EVEL	OF SER		CALCU	JLATIO	NS					
MOVE	-	# OF		1	SCE	ENARIO	VOLUM	<u>ES</u>			3	SCENARIO	V/C RATIO	<u>s</u>	
MENT	S	LANES	CAPACITY	1	2	3	4			1	2	3	4	1	
NBL		1	1600	67	67	67	67			0.042 *	0.042 *	0.042 *	0.042 *		
NBT NBR	(a)		3200	63	579 63	634 66	661 66			0.173	0.181	0.198	0.207		
	()														
SBL		1	1600	44	44	55	55			0.028	0.028	0.034	0.034		
SBT	(b)	2	3200	697	713	765	781			0.218 *	0.223 *	0.239 *	0.244 *		
SBR	(D)	'	1000	56	00	01	04			0.030	0.036	0.036	0.040		
EBL		1	1600	92	97	94	99			0.058	0.061	0.059	0.062		
EBT		2	3200	135	135	136	136			0.063 *	0.063 *	0.063 *	0.063 *		
EBR	(c)	0	0	66	66	66	66			-	-	-	-		
WBL		1	1600	52	52	60	60			0.033 *	0.033 *	0.038 *	0.038 *		
WBT		2	3200	84	84	86	86			0.034	0.034	0.039	0.039		
WBR	(d)	0	0	24	24	39	39			-	-	-	-		
							LOS	T TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION: 0.456 0.4										0.461	0.482	0.487			
				S	CENAR	IO LEVE	LOFSE	RVICE	:	Α	Α	A	A		
NOTE	S:	<b>D</b> =0-	( ) 000(												
		RTOR:	(a) 20% (b) 10% (c) 0% (d) 0%												
	Printed:	03/27/24													
EXIST	EXISTING: < THIS COMPARES TO CONDITION (A)														

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#24017 - INTERSE COUNT E TIME PEH N/S STRE E/W STR CONTRO	SEAR ECTIO DATE: RIOD: EET: REET: DL TYF	<b>PE</b> :	DING REMODEL CITY UTILIZATIO 01/18/2023 PM PEAK HOU BROADWAY COOK STREE SIGNAL	UR	CT KSHEET	Γ									REF:	06_PM
			NO			SOU	FFIC V		E SUN			W/ES				
VOLUME	S		L	Т	R	L 500	T	R	L T R			L	T	R		
(A) E	VISTIN	IC:	125	913	57	74	765	100	1/2	198	99	140	256	111		
(B) P	ROJE	CT-ADDED	): <b>0</b>	73	0	0	73	13	13	0	0	0	0	0		
(C) C	UMUL	ATIVE:	125	995	64	85	858	103	145	200	99	145	257	126		
GEOMETRICS																
			NO	RTH BO	UND	SOU	ТН ВОІ	JND	EAS	T BOUI	ND	WES	T BOUND			
LANE GE	EOME	TRICS		L TT	R		L TT	R		LTTF	२		L T TR			
							TRAFF		NARIC	S						
SCENAR SCENAR SCENAR SCENAR	SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B) SCENARIO 3 = SHORT-TERM CUMULATIVE (C) SCENARIO 4 = SHORT-TERM CUMULATIVE + PROJECT VOLUMES (B+C)															
					L	EVEL	OF SEF		CALCU	ILATIO	NS					
MOVE-		# OF				SCE	ENARIO	VOLUM	ES			0	SCENARIO	V/C RATIO	<u>s</u>	
MENTS		LANES	CAPACIT	Ϋ́	1	2	3	4			1	2	3	4		
NBL		1	1600		125	125	125	125			0.078	0.078	0.078	0.078		
NBT		2	3200		913	986	995	1068			0.285 *	0.308 *	0.311 *	0.334 *		
NBR (a	a)	1	1600		40	40	45	45			0.025	0.025	0.028	0.028		
SBI		1	1600		74	74	85	85			0.046 *	0.046 *	0.053 *	0.053 *		
SBT		2	3200		765	838	858	931			0.239	0.262	0.268	0.291		
SBR (b	b)	1	1600		70	79	72	81			0.044	0.049	0.045	0.051		
											0.000 t	0.007 ±				
EBL		1	1600		142	155 108	145 200	158 200			0.089 *	0.097 *	0.091 *	0.099 *		
EBR (c	c)	0	0		99	99	200 99	200 99			-	-	-	-		
WBL		1	1600		140	140	145	145			0.088	0.088	0.091	0.091		
WBT WBR (c	-1)	2	3200		256 111	256 111	257 126	257 126			0.115 *	0.115 *	0.120 *	0.120 *		
	<i></i>	0	Ů				120	120			-	-	-	-		
								LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
				TOT		SECTIO			TII 1747		0.625	0.666	0.675	0.700		
				1017	S	CENAR	IO LEVE	L OF SI	ERVICE	:	B	B	B	C.700		
NOTES:											I	I	l	I	1	
		RTOR:	(a) 30% (b) 30% (c) 0% (d) 0%													
F	Printed:	03/27/24														
EXISTINO	EXISTING:THIS COMPARES TO CONDITION (A)															

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#24017 - SEARS BUILDING REMODEL PROJECTRIINTERSECTION CAPACITY UTILIZATION WORKSHEETCOUNT DATE:01/18/2023TIME PERIOD:AM PEAK HOURN/S STREET:MILLER STREETE/W STREET:COOK STREETCOOK STREETCONTROL TYPE:SIGNALSIGNALCONTROL TYPE:SIGNAL													REF:	07_AM	
					TRA	FFIC V	OLUM	E SUN	IMARY						
	-s			BOUND	SOU	T BOI	JND R	EAS	T BOU	ND R	I WES	T BOUND	R		
(A) F		IG.	77 29	8 24	8	341	97	82 51 76			49	12			
(B) F	PROJE	CT-ADDED	): <b>0 1</b> (	) 0	1	6	0	0	0	0	0	0	3		
(C) (C)	CUMUL	ATIVE:	84 32	1 24	10	357	103	94	55	84	49	64	12		
GEOMETRICS															
			NORTH	BOUND	SOU	ТН ВОІ	JND	EAS	T BOU	ND	WES	T BOUND	)		
LANE GE	EOME	TRICS	L 1	TR		LTF	2		LT	R		LTR			
						TRAFF	IC SCE	NARIO	DS						
SCENAF SCENAF SCENAF SCENAF	SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B) SCENARIO 3 = SHORT-TERM CUMULATIVE (C) SCENARIO 4 = SHORT-TERM CUMULATIVE + PROJECT VOLUMES (B+C)														
				L	EVEL	OF SEF	RVICE	CALCU	ILATIO	NS					
MOVE-		# OF	CAPACITY	1	<u>SCE</u> 2	ENARIO 3	VOLUM	I <u>ES</u>		1	2	<u>s</u>			
NBI		1	1600	77	77	84	84			0.048 *	0.048 *	0.053 *	0.053 *		
NBT		2	3200	298	308	321	331			0.101	0.1040	0.108	0.000		
NBR (	a)	0	0	24	24	24	24			-	-	-	-		
SBI		1	1600	9	0	10	11			0.005	0.006	0.006	0.007		
SBL			1600	341	347	357	363			0.003	0.000 *	0.223 *	0.227 *		
SBR (	Ъ)	1	1600	87	87	93	93			0.054	0.054	0.058	0.058		
EDI		1	1600	00	00	04	04			0.051 *	0.051 *	0.050 *	0.050 *		
EBL			1600	51	₀∠ 51	94 55	94 55			0.031	0.031	0.039	0.039		
EBR (	(c)	1	1600	68	68	76	76			0.043	0.043	0.048	0.048		
W/DI			0	40	40	40	40								
WBL			1600	49 62	49 62	49 64	49 64			- 0.074 *	- 0.076 *	- 0.076 *	0.078 *		
WBR (	d)	0	0	8	11	8	11			-	-	-	-		
		1	I				LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY U SCENARIO LEVEL OF S									FION:	0.486 A	0.492 A	0.511 A	0.517 A		
NOTES:															
		RTOR:	<ul> <li>(a) 0%</li> <li>(b) 10%</li> <li>(c) 10%</li> <li>(d) 0%</li> </ul>												
	Printed:	03/27/24													
EXISTIN	EXISTING: < THIS COMPARES TO CONDITION (A)														

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)

#24017 - SEARS BUILDING REMODEL PROJECT         INTERSECTION CAPACITY UTILIZATION WORKSHEET         COUNT DATE:       01/18/2023         TIME PERIOD:       PM PEAK HOUR         N/S STREET:       MILLER STREET         E/W STREET:       COOK STREET         CONTROL TYPE:       SIGNAL											REF:	07_PM				
						TRA	FFIC V	OLUM	E SUM	IMARY						
VOLUMES	2		NOF	TH BOU	IND P	SOU	TH BOI	JND	EAS	T BOU	ND	WES	T BOUND	P		
		0.	L	400		40	400	<u> </u>	470		170					
(A) EX (B) PR		G: T-ADDED	): 0	400 25	0	19 7	422 25	0	0	0	0	28 0	0	о 8		
(C) CU	JMULA	ATIVE:	181	504	22	21	449	159	180	68	182	28	145	8		
GEOMETRICS																
			NOR	RTH BOU	IND	SOU	TH BOI	JND	EAS	T BOU	ND	WES		)		
LANE GEO	ОМЕТ	RICS		LTTR			LTF	<b>२</b>		LT	R		LTR			
							TRAFF	IC SCE	NARIC	os						
SCENARIO SCENARIO SCENARIO SCENARIO	SCENARIO 1 = EXISTING VOLUMES (A) SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B) SCENARIO 3 = SHORT-TERM CUMULATIVE (C) SCENARIO 4 = SHORT-TERM CUMULATIVE + PROJECT VOLUMES (B+C)															
					L	EVEL	OF SEF	RVICE	CALCU	ILATIO	NS					
MOVE-		# OF	CADACITY	/	4	SCE	ENARIO	VOLUN	IES		1	2	SCENARIO	V/C RATIO	<u>s</u>	
NDI					170	170	<b>3</b>	4			0.106 *	2	<b>3</b>	4		
NBL		2	3200		486	511	504	529			0.159	0.100	0.164	0.172		
NBR (a)		0	0		22	22	22	22			-	-	-	-		
CDI		4	1000		40	00	04	00			0.040	0.010	0.040	0.040		
SBL		1	1600		19 422	26 447	21 449	28 474			0.012	0.016	0.013	0.018		
SBR (b)		1	1600		104	104	111	111			0.065	0.065	0.069	0.069		
					470							0 400 t				
EBL		1	1600		173 67	173 67	180 68	180			0.108 *	0.108 *	0.113 *	0.113 *		
EBR (c)		1	1600		125	125	127	127			0.078	0.078	0.079	0.079		
WBL		0	0		28	28	28 145	28			-	-	-	-		
WBR (d)		0	0		8	16	8	145			-	-	-	-		
											0.400 *	0.400 *	0.400 *	0.400 +		
								LOS	ST TIME:		0.100 *	0.100 *	0.100 *	0.100 *		
				ΤΟΤΑΙ	L INTER	SECTIC		ACITY L		FION:	0.689	0.709	0.720	0.740		
					S	CENAR	IO LEVE	L OF S	ERVICE	:	В	С	C	c		
NOTES:											1		1	L	l	
		RTOR:	(a) 0% (b) 30% (c) 30% (d) 0%													
Pri	inted:	03/27/24														
EXISTING: < THIS COMPARES TO CONDITION (A)																

SCENARIO 2 = EXISTING + PROJECT VOLUMES(A+B)

SCENARIO 3 = SHORT-TERM CUMULATIVE (C)